

STATE		FEDERAL AID		STATE	SHEET
STATE	ROUTE	PROJECT	ROUTE	PROJECT	NO.
VA.	674		674	0674-029-6134	- 1
NBIS 1	Numbe	er: 00000000006830	UPC	No. 106819	
Feder	al Ov	ersight Code: NEO		Construction Scour Code:	_

The original approved sheet, including original signatures, is filed in the VDOT Central Office. Any misuse of electronic files is illegal. Violators will be prosecuted to the full extent of the applicable laws.

Construction: Virginia Department of Transportation Road and Bridge Specifications, 2016.

Design: AASHTO LRFD Bridge Design Specifications, 8th Edition, 2017; and VDOT Modifications.

Standards: Virginia Department of Transportation Road and Bridge Standards, 2016; including all current revisions.

These plans are incomplete unless accompanied by the Supplemental Specifications and Special Provisions included in the contract docu-

Design loading includes 20 psf allowance for construction tolerances and construction methods.

Prestressed concrete in the prestressed members shall be ${\it Class}$ A5 having a minimum compressive cylinder strength at 28 days equal to 6000 psi and a minimum compressive cylinder strength at time of release of strands equal to 4000 psi.

Concrete in railings shall be Class A4. Concrete in abutments shall be Class A3.

All reinforcing steel shall be deformed and shall conform to ASTM A615 Grade 60 except for reinforcing steel noted as CRR (corrosion resistant reinforcement) which shall conform to Section 223 of the Specifications. All reinforcing bar dimensions on the detailed drawings are to centers of bars except where otherwise noted and are subject to fabrication and construction tolerances.

Corrosion resistant reinforcing (CRR) steels shall conform to one or more of the three Classes listed in Section 223 of the Specifications. The Class(es) of CRR steel(s) required on this project is/are noted on plan sheets and in the reinforcing steel schedule. CRR Steel, Class II or Class III, may be substituted for Class II. CRR Steel, Class III, may be substituted for Class II.

Prestressing strands shall be uncoated, seven-wire, low-relaxation steel strands conforming to ASTM A416 Grade 270.

The Bridge Date Plate shall be installed in accordance with VDOT's Road and Bridge Standards and obtained from the District Structure and Bridge Office.

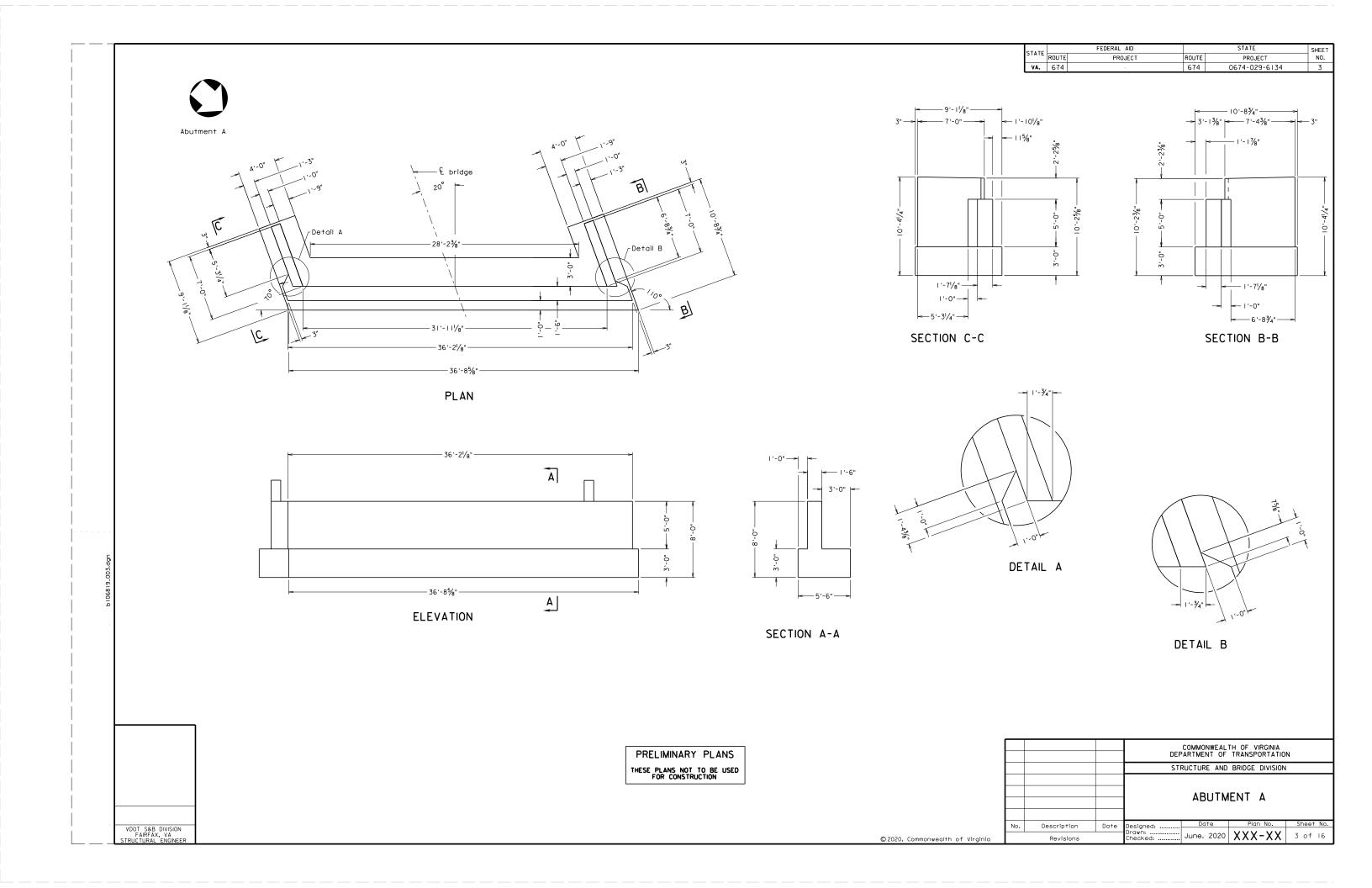


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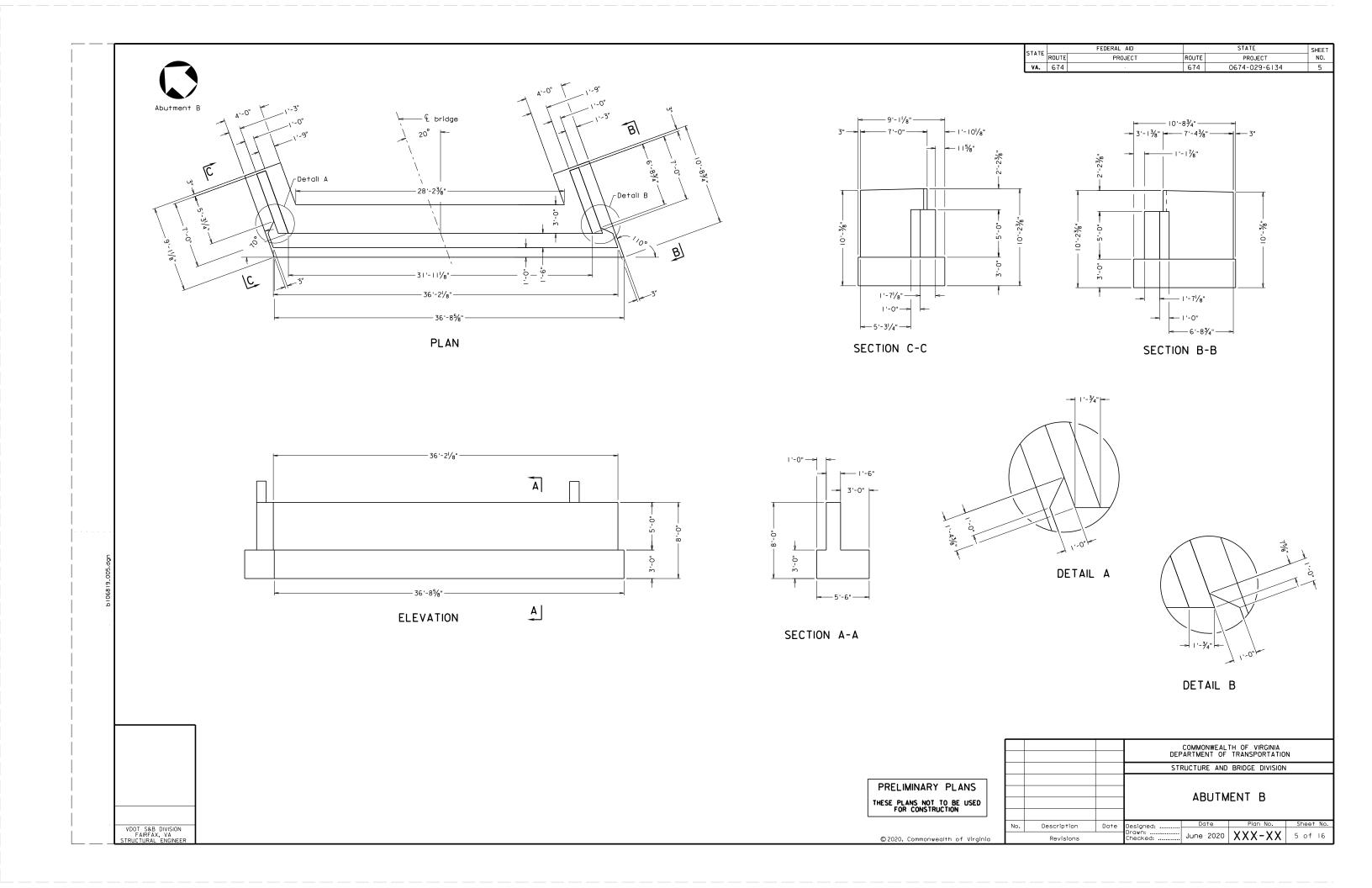
DEPARTMENT OF TRANSPORTATION PROPOSED BRIDGE ON RTE. 674 OVER PINEY RUN FAIRFAX COUNTY - 0.3 MI N.E. OF RTE 007 0674-029-6134

Recommended for Approv	ral:	
Approved:		
	District Maintenance Manager	Date
		XXX-XX
Date: June 05, 2020	© 2020, Commonwealth of Virginia	Sheet I of I

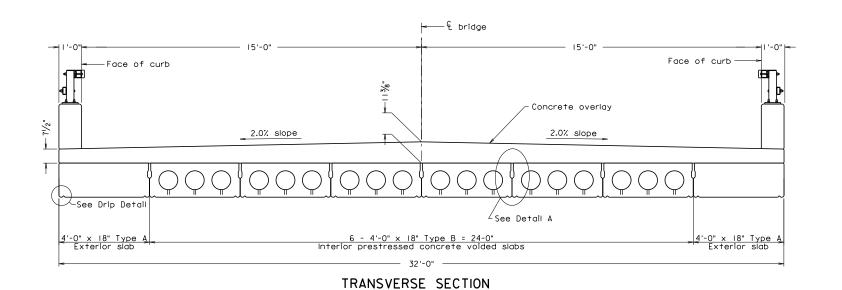
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NOTES:

The bridge and roadway widths shown are nominal. Actual widths may vary due to fabrication and construction (gaps between slabs) tolerances.

Concrete for the overlay shall be Low Shrinkage Class A4 Modified having a minimum 28 day compressive strength of 4000 psi. Payment for the concrete overlay shall be made at the unit price for Concrete Class A4.

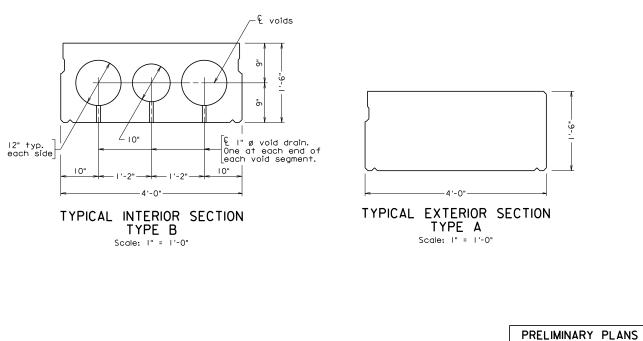
All reinforcing bars in concrete overlay shall be Corrosion Resistant Reinforcing steel, Class \dots

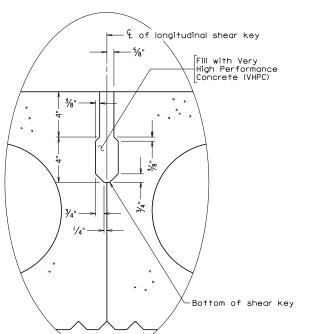
Top surfaces of all slabs shall be a clean concrete surface, free of laitance, with surface intentionally roughened to an amplitude of $\frac{1}{4}$ ".

Very High Performance Concrete shall be furnished, placed and paid for in accordance with the current VDOT Special Provision for Shear Keys and Blockouts Between Adjacent Members.

Casting of parapets shall not be done until all grouting of keys are completed and the VHPC has reached a minimum strength of 4000 psi.

For waterproofing details, see sheet __.







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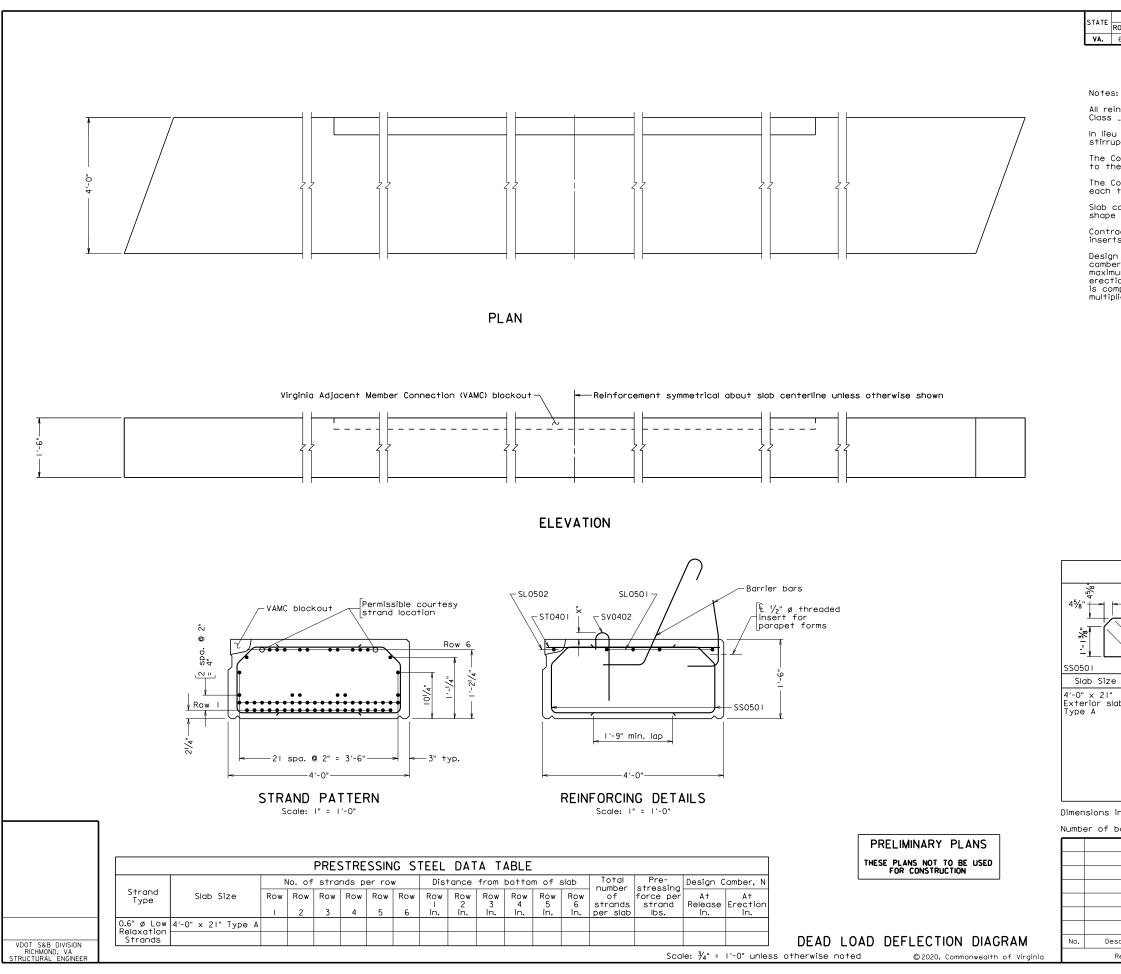
DRIP DETAIL

Typ. each side of each member

PRELIMINARY PLANS
THESE PLANS NOT TO BE USED FOR CONSTRUCTION

Scale: $\frac{1}{2}$ " = 1'-0" unless otherwise noted

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All reinforcing bars shall be Corrosion Resistant Reinforcing Steel, Class $_{\mbox{\scriptsize --}}.$

In lieu of spilcing two reinforcing bars to form each stirrup, the stirrup may be made from one single bar.

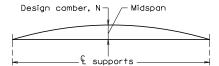
The Contractor may submit an alternate prestressing strand pattern to the Engineer for approval. $\begin{tabular}{ll} \hline \end{tabular}$

The Contractor has the option of stressing two courtesy strands each to $8,\!000$ lbs at the locations indicated.

Slab corners damaged during construction shall be restored to their shape as shown on the plans by an approved epoxy mortar.

Contractor shall determine location and spacing of $\frac{1}{2}$ " ø threaded

Design and detailing of these plans are based on the design camber(s) at erection (see Prestressing Steel Data Table) and the maximum tolerance for camber differential from design camber at erection indicated in the Specifications. Design camber at erection is computed using Precast/Prestressed Concrete Institute (PCI) multipliers.



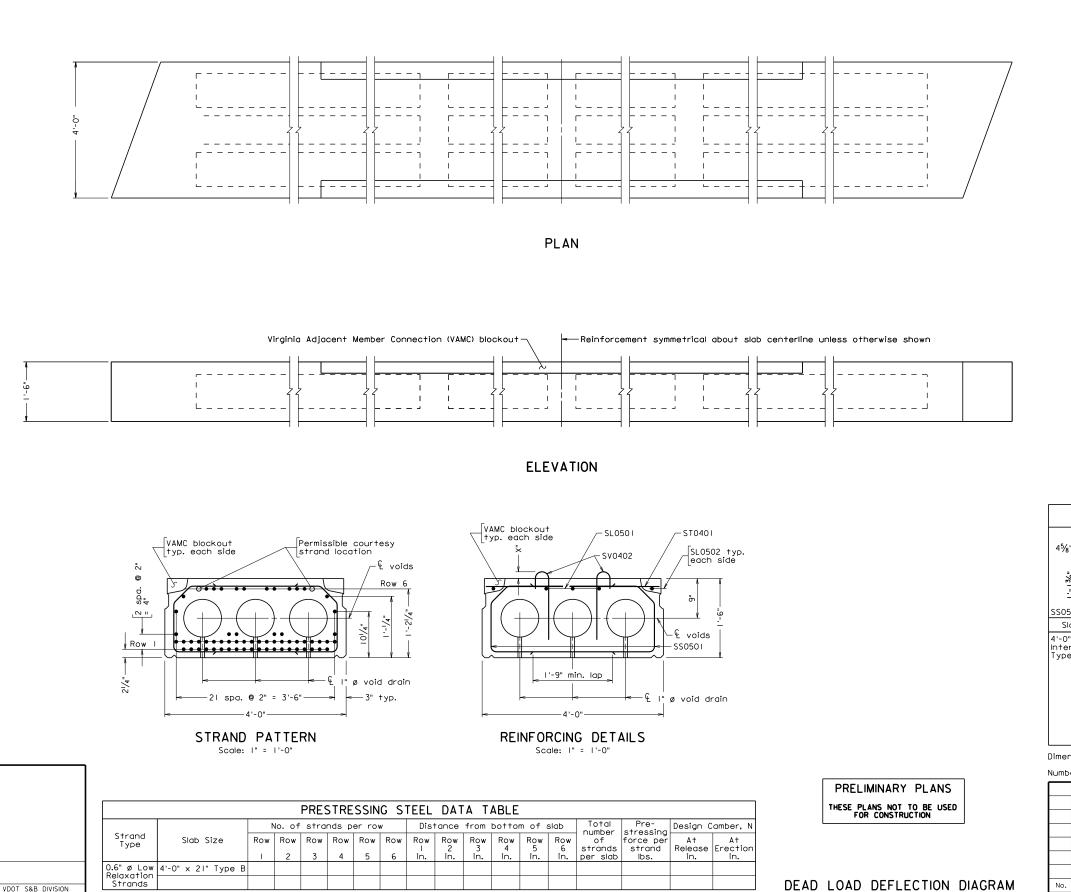
CAMBER DIAGRAM

REINFORCING STEEL SCHEDULE **--**3'-61∕2"· SH040 I SV0501 SV0402 Size Pin ø Length Slab Size Mark No. Location 4'-0" x 21" Exterior slab 5'-4" End horizontal SH0401 SL0501 Top longitudinal SL0502 **#**5 Top longitudinal SS0501 **#**5 21/2" 6'-7" Stirrup ST0401 3'-10" Top transverse **#**5 3¾" 3'-8" End vertical SV0501 #4 SV0402 3" Composite vertical

Dimensions in bending diagram are out-to-out of bars.

Number of bars shown in table are per slab per slab type.

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Scale: $\frac{3}{4}$ " = 1'-0" unless otherwise noted

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VDOT S&B DIVISION FAIRFAX, VA

Notes:

All reinforcing bars shall be Corrosion Resistant Reinforcing Steel, Class \ldots

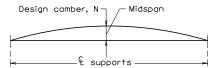
In lieu of spilcing two reinforcing bars to form each stirrup, the stirrup may be made from one single bar.

The Contractor may submit an alternate prestressing strand pattern to the Engineer for approval.

The Contractor has the option of stressing two courtesy strands each to $8,000\ \mathrm{lbs}$ at the locations indicated.

Slab corners damaged during construction shall be restored to their shape as shown on the plans by an approved epoxy mortar.

Design and detailing of these plans are based on the design camber(s) at erection (see Prestressing Steel Data Table) and the maximum tolerance for camber differential from design camber at erection indicated in the Specifications. Design camber at erection is computed using Precast/Prestressed Concrete Institute (PCI) multipliers.



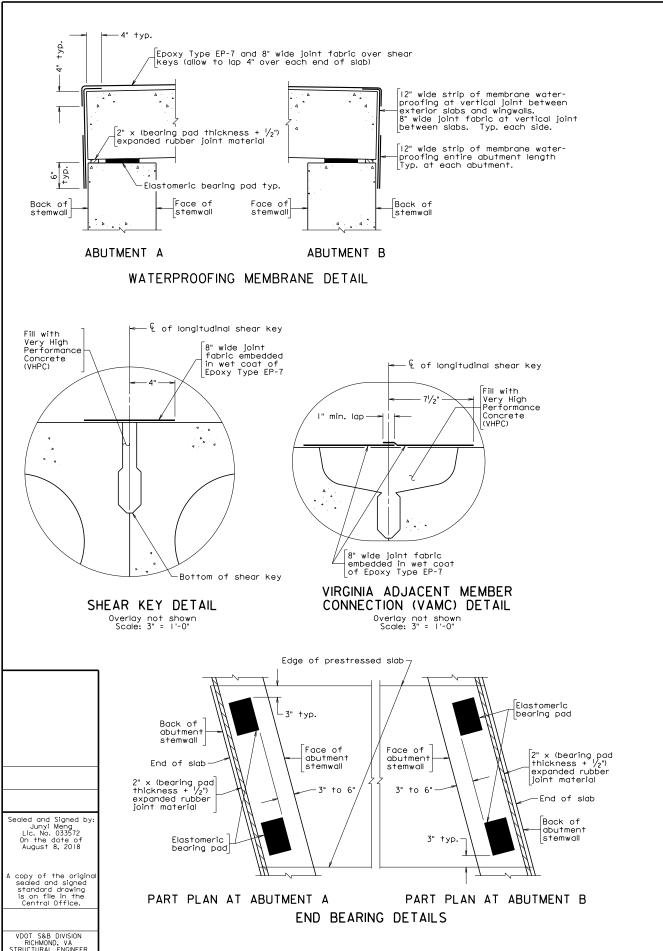
CAMBER DIAGRAM

REINFORCING STEEL SCHEDULE --3'-6¹/2" SS0501 SH040 I SV050 SV0402 Size Pin ø Length Slab Size Mark No. Location 4'-0" × 21" SH0401 5'-4" End horizontal Interior slab Type B SL0501 Top longitudinal SL0502 **#**5 Top longitudinal SS0501 **#**5 21/2" 6'-7" Stirrup ST0401 3'-10" Top transverse **#**5 33/4" SV0501 3'-8" End vertical #4 SV0402 3" Composite vertical

Dimensions in bending diagram are out-to-out of bars.

Number of bars shown in table are per slab per slab type.

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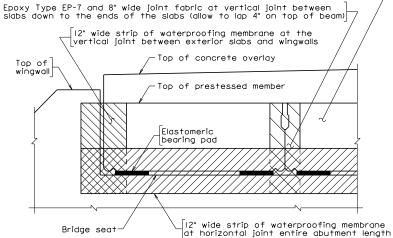
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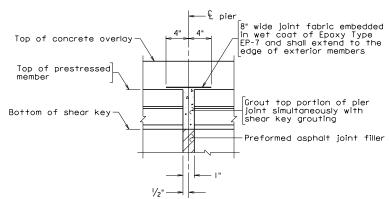
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Waterproofing, typ. (Type EP-3B and EP-3T) per Sec. 405.05(g). This portion of waterproofing shall be applied at the plant and included in the cost of the prestressed members.



END OF SLAB VIEW AT ABUTMENTS



SECTION THRU PIER JOINTS

Typical all pier joints Normal to joint Not to Scale

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Notes:

Details shown for 4'-0" x 18" prestressed concrete voided slab. Details similar for other widths and depths.

The Contractor shall adjust bearing pads or bridge seats as directed by the Engineer where prestressed members are not in full bearing with the pads or where the pads are not in full bearing with the seats. Cost of any adjustment shall be included in other bid items.

Procedure for sealing shear keys and pier joint utilizing Epoxy EP-7 and joint fabric:

Surface preparation shall be in accordance with Section 416 and application guidelines, Epoxy EP-7 and sand shall be in accordance with Section 431 of the Specifications. All Epoxy Type EP-7 shall be field applied (i.e., not applied at the plant).

I. Prepare deck surface.

2. Apply a coat of epoxy to the slab ends at the shear keys and then set the joint fabric into the wet epoxy. Allow enough joint fabric to provide for a 4" min. lap on top of slabs. Apply additional epoxy over the joint fabric to thoroughly wet the joint fabric. Remove any air pockets under the joint fabric using a short nap paint roller.

3. Apply the first coat of epoxy to slab surface over the grouted shear keys and set the joint fabric (continuous over pier joints) into the wet epoxy. Allow enough joint fabric to provide for a 4" min. lap down the slab ends. Apply additional epoxy over the joint fabric to thoroughly wet the joint fabric and provide sufficient free epoxy to engage the sand. Remove any air pockets under the joint fabric using a short nap paint roller. Apply sand and allow epoxy and sand to cure and then remove loose sand.

4. Apply the first coat of epoxy over pier joint and set the joint fabric into the wet epoxy. Apply additional epoxy over the joint fabric to thoroughly wet the joint fabric and provide sufficient free epoxy to engage the sand. Remove any air pockets under the joint fabric using a short nap paint roller. Apply sand and allow epoxy and sand to cure and then remove loose sand.

5. Apply a second coat of epoxy and sand over shear keys and pier joint. Remove loose sand after epoxy has cured.

6. Any epoxy spills outside the limits of the shear key treatment shall be covered with sand while the epoxy is still wet.

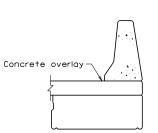
The cost of Epoxy Type EP-7, sand and joint fabric shall be included in the cost of the prestressed members.

Joint fabric from the VDOT Special Products Evaluation List under Joint Fabrics shall be used.

Notes for End of Slab Membrane Waterproofing:

The membrane shall be applied to the end of slabs at abutments at the vertical joint between exterior slabs and wingwalls and the horizontal joint as shown in END OF SLAB VIEW AT ABUTMENTS.

The membrane shall consist of a Type III preformed membrane system in accordance with Section 429 of the Specifications. The cost of the membrane shall be included in the cost of the prestressed members.



SECTION THRU PARAPET

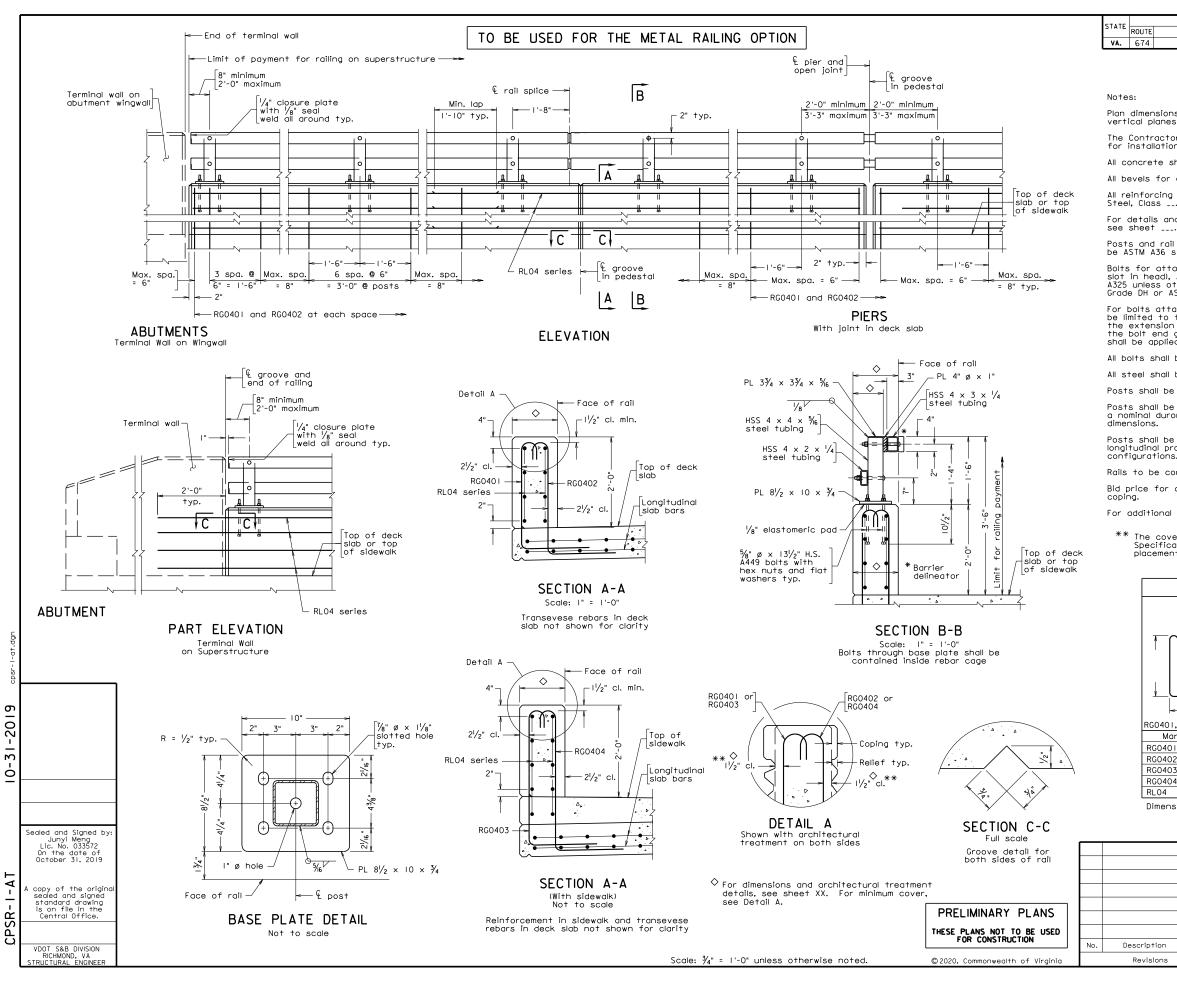
Scale: 1/2" = 1'-0"

Scale: I" = I'-0" unless otherwise shown ©2020, Commonwealth of Virginia

PRELIMINARY PLANS

THESE PLANS NOT TO BE USED FOR CONSTRUCTION

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Plan dimensions shown are measured in the respective horizontal and vertical planes.

The Contractor shall determine all dimensions and details necessary for installation.

All concrete shall be Low Shrinkage Class A4 Modified.

All bevels for concrete shall be 3/4".

All reinforcing steel shall be Corrosion Resistant Reinforcing Steel, Class __.

For details and reinforcing steel schedule of terminal wall, see sheet ____.

Posts and rail members shall be ASTM A500 Grade B steel. Plates shall be ASTM A36 steel. Steel pipe sleeves shall be ASTM A53.

Bolts for attaching rails to post are $\frac{3}{4}$ " diameter round head (with slot in head), ASTM A449. All other bolts shall be ASTM F3125 Grade A325 unless otherwise indicated in the details. Nuts shall be ASTM A563 Grade DH or ASTM A194 Grade 2H. Washers shall be ASTM F436.

For bolts attaching rails to posts, bolt extensions beyond nut shall be limited to the smaller of one and a half finishing turns or $\frac{1}{4}$ ". If the extension is longer, excess shall be cut off and the edges of the bolt end ground so that no sharp edges remain. Cold galvanizing shall be applied to damaged galvanized areas.

All bolts shall be snua tightened.

All steel shall be hot dip galvanized.

Posts shall be equally spaced within a span. Maximum spacing is 6'-8".

Posts shall be seated on neoprene pads $1\!/_8{}^{\rm m}$ minimum thickness, having a nominal durometer hardness of 60. Pads shall conform to post base dimensions.

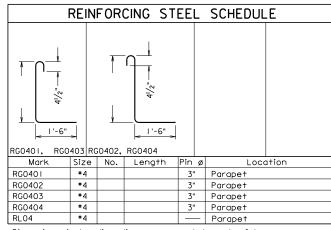
Posts shall be vertical in transverse direction and normal to longitudinal profile grade. Cut bottom of posts to meet these configurations.

Rails to be continuous over a minimum of 3 posts before splicing.

 $\ensuremath{\mathsf{Bid}}$ price for architectural treatment includes concrete in relief and coping.

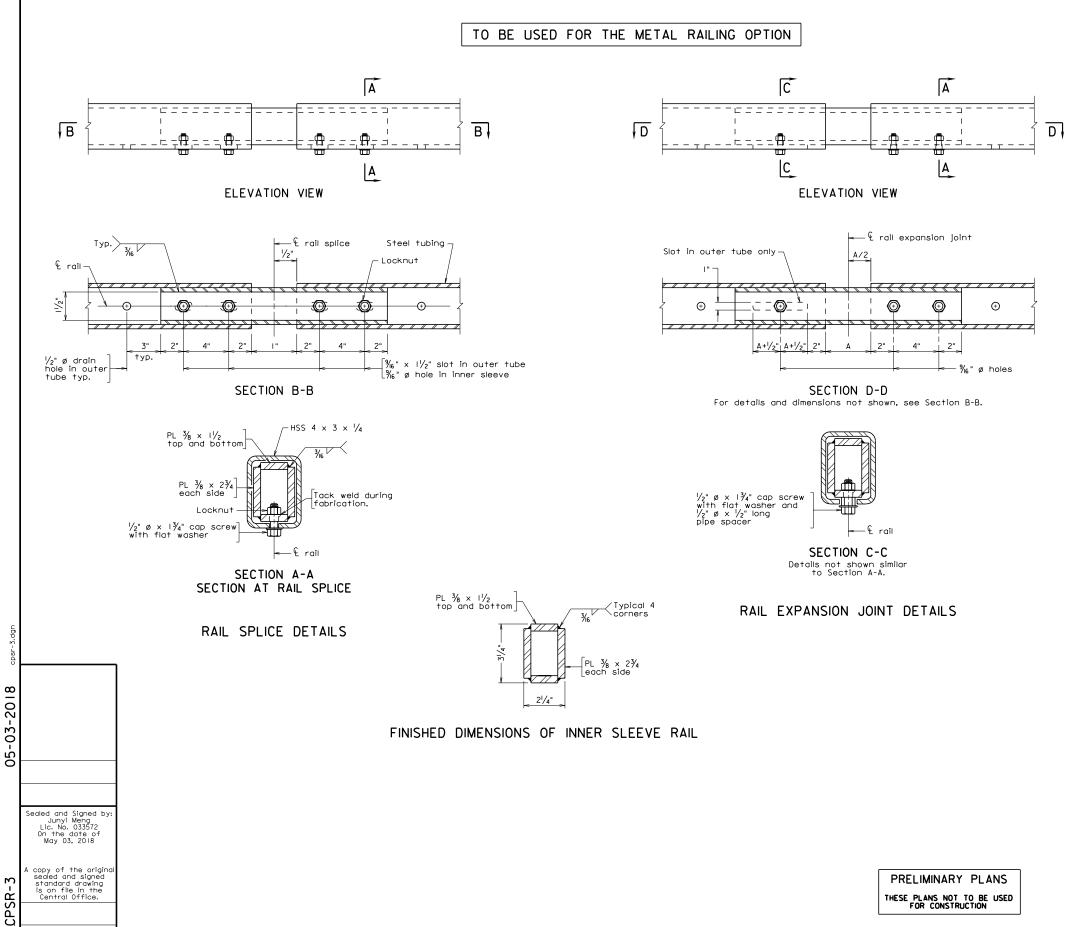
For additional notes, see sheet__.

** The cover tolerance referenced in the VDOT Road and Bridge Specifications as -0" to +1/2" is shifted to -1/4" to +1/4" for placement of the RGO4 series bars.



Dimensions in bending diagram are out-to-out of bars.

			COMMONWEALTH OF VIRGINIA DEPARTMENT OF TRANSPORTATION
			STRUCTURE AND BRIDGE DIVISION
			42"-CPSR RAILING WITH ARCHITECTURAL TREATMENT (CPSR-I-AT)
No.	Description	Date	Designed: \$&B. DIV Date Plan No. Sheet No. Drawn:\$\frac{88}{28}\frac{11}{21}\frac{1}{21}
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Notes (cont'd):

Drain holes shall be 1/2" diameter and shall be provided in all rails approximately half-way between posts except at open joints near pier(s). Drain holes shall be provided at each end of rail.

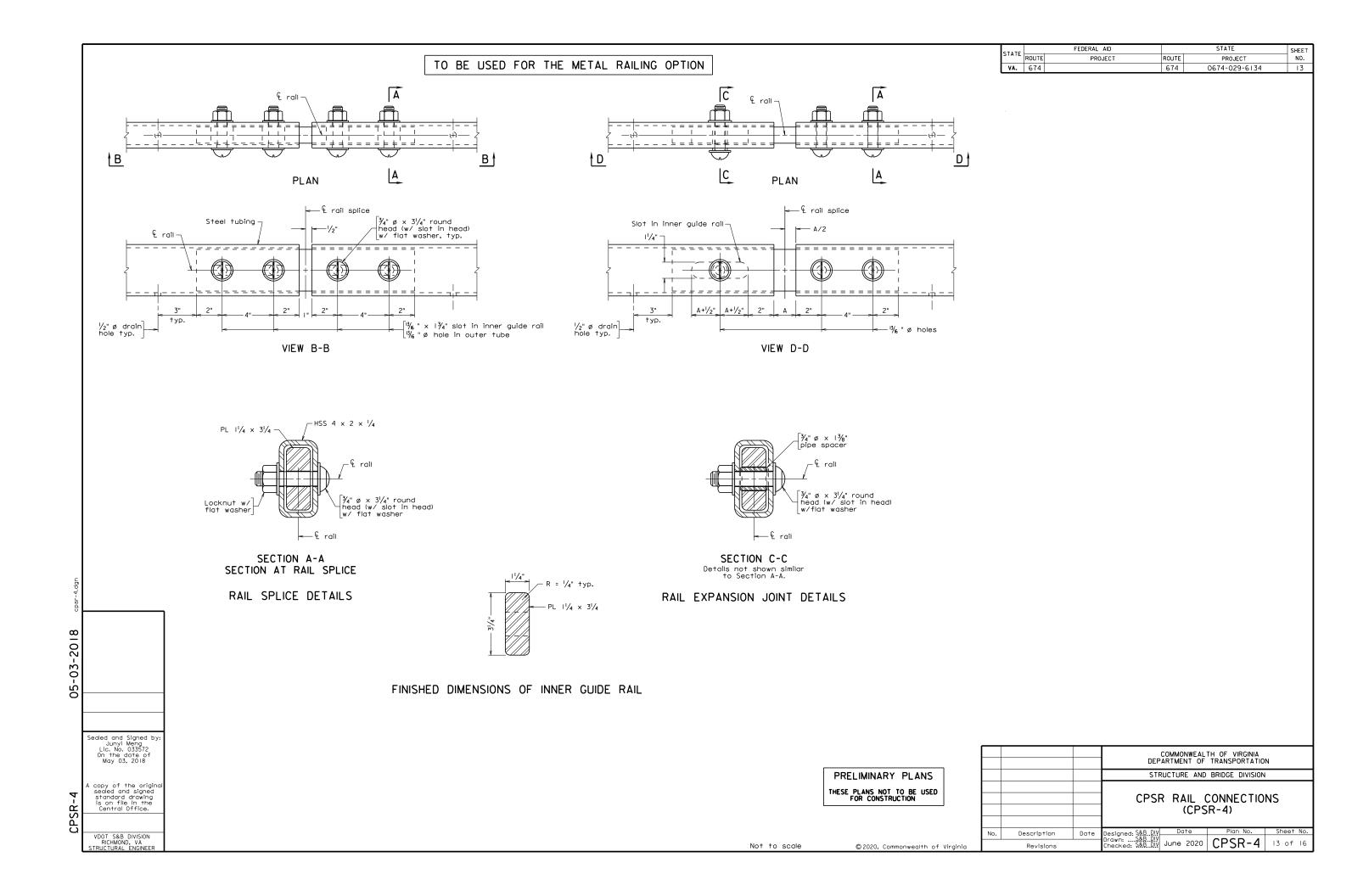
Barrier delineator size, color, and spacing shall be in accordance with the Specifications.

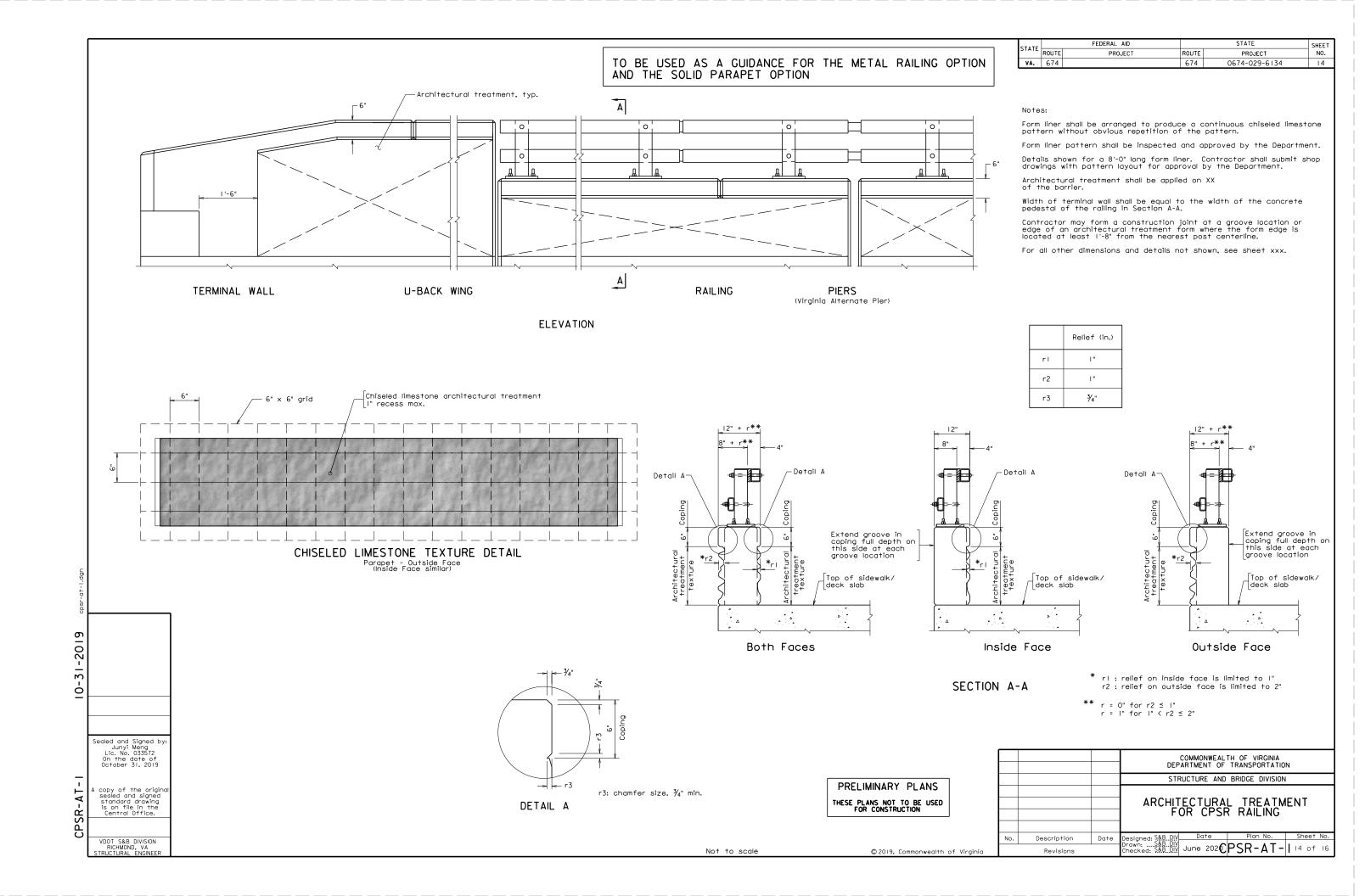
Maximum spacing of grooves in pedestal shall be limited to 3 x post spacing, shall be centered between posts and shall be no closer than 10'-0" to ends of pedestal. Where deck slab is continuous over a pier, a groove in pedestal shall be provided at the pier.

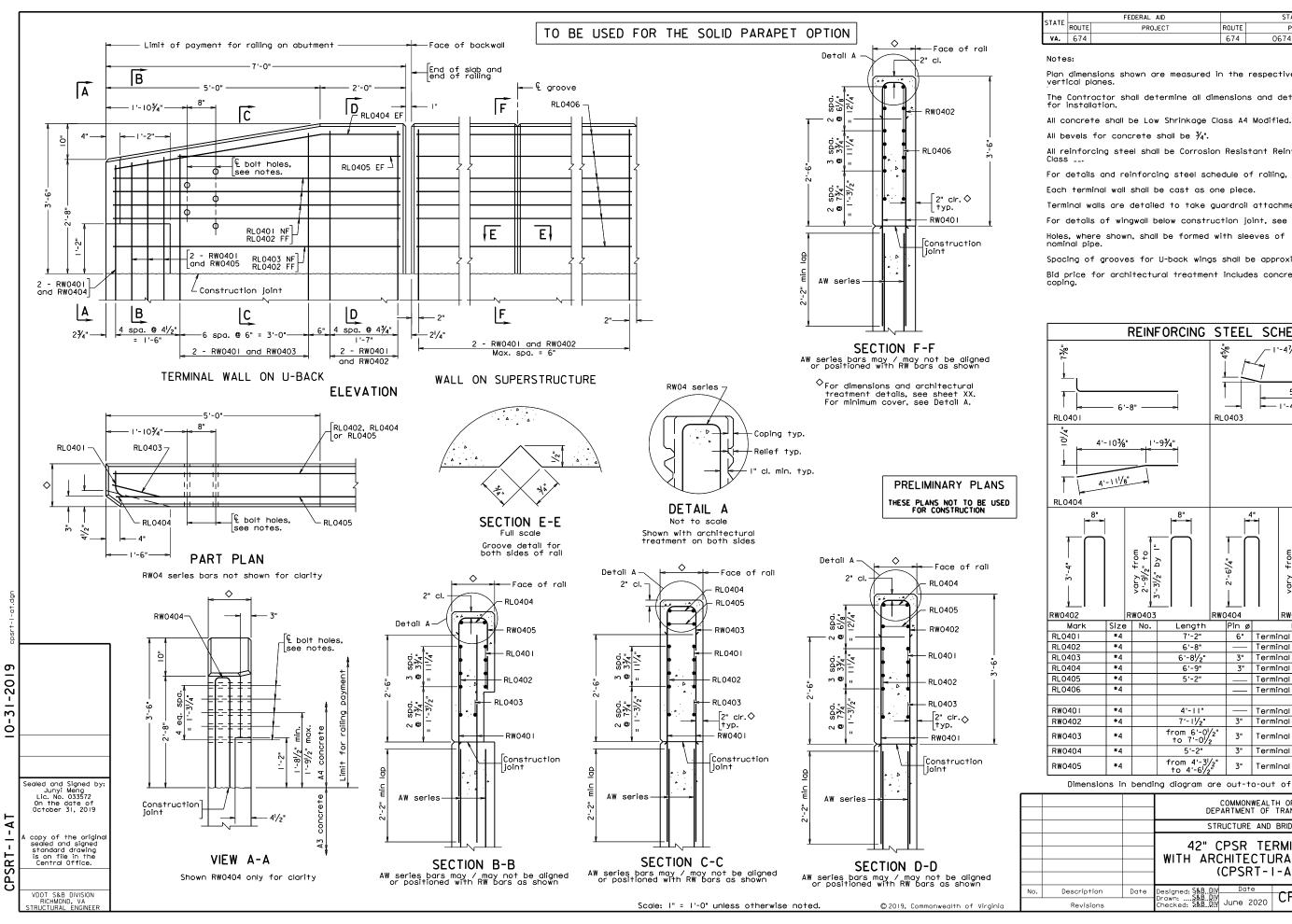
Alternate details for inner sleeve rail fabrication and bolted connection to outer tube may be submitted, but only used if approved by the Structure and Bridge Division Engineering Services Program Area. No thru-bolt connections will be approved.

Bid item for railing shall include rails, rail posts, bearing pads, bolts, anchor assemblies, sleeves, barrier delineators, grounding materials and other associated metal parts as shown on the plans. Also included is concrete noted in the plans and reinforcing steel indicated in the reinforcing steel schedule.

				COMMONWEALTH OF VIRGINIA DEPARTMENT OF TRANSPORTATION
				STRUCTURE AND BRIDGE DIVISION
MINARY PLANS				CPSR RAIL CONNECTIONS
ANS NOT TO BE USED CONSTRUCTION				AND NOTES (CPSR-3)
	No.	Description	Date	Designed: \$&BDiV Date Plan No. Sheet No.
© 2020, Commonwealth of Virginia		Revisions		Designed: \$&B. DIY







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Plan dimensions shown are measured in the respective horizontal and

The Contractor shall determine all dimensions and details necessary for installation.

All reinforcing steel shall be Corrosion Resistant Reinforcing Steel.

For details and reinforcing steel schedule of railing, see sheet ___.

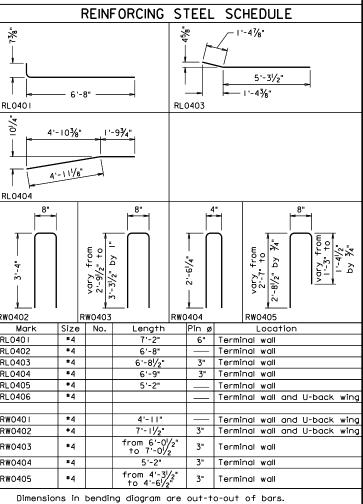
Terminal walls are detailed to take guardrail attachment for MGS.

For details of winawall below construction joint, see abutment details.

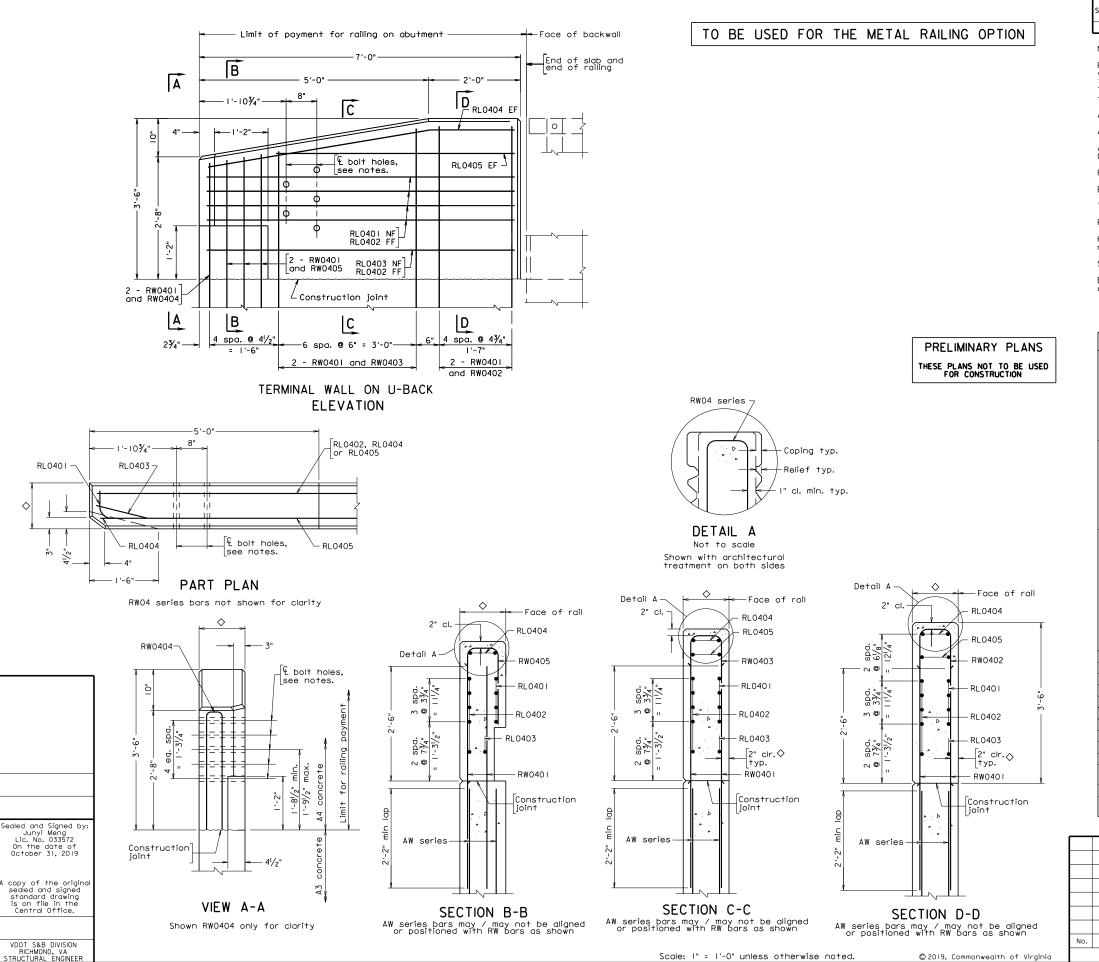
Holes, where shown, shall be formed with sleeves of $I^{1}/_{4}$ " diameter

Spacing of grooves for U-back wings shall be approximately 8'-0".

Bid price for architectural treatment includes concrete in relief and



			COMMONWEALTH OF VIRGINIA DEPARTMENT OF TRANSPORTATION
			STRUCTURE AND BRIDGE DIVISION
			42" CPSR TERMINAL WALL WITH ARCHITECTURAL TREATMENT (CPSRT-I-AT) -I
No.	Description	Date	Designed: \$&BDIV Date Plan No. Sheet No.
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Notes:

Plan dimensions shown are measured in the respective horizontal and vertical planes.

The Contractor shall determine all dimensions and details necessary for installation. $\label{eq:contractor} % \begin{subarray}{ll} \hline \end{subarray} % \begin{subarr$

All concrete shall be Low Shrinkage Class A4 Modified.

All bevels for concrete shall be $\frac{3}{4}$ ".

All reinforcing steel shall be Corrosion Resistant Reinforcing Steel, Class \hdots

For details and reinforcing steel schedule of railing, see sheet __. Each terminal wall shall be cast as one piece.

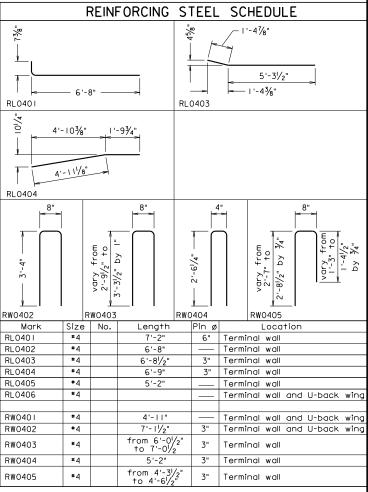
Terminal walls are detailed to take guardrail attachment for MGS.

For details of wingwall below construction joint, see abutment details.

Holes, where shown, shall be formed with sleeves of $1^{1}/_{4}^{1}$ diameter nominal pipe.

Spacing of grooves for U-back wings shall be approximately 8'-0".

Bid price for architectural treatment includes concrete in relief and coping.



Dimensions in bending diagram are out-to-out of bars.

			COMMONWEALTH OF VIRGINIA DEPARTMENT OF TRANSPORTATION STRUCTURE AND BRIDGE DIVISION
			42" CPSR TERMINAL WALL WITH ARCHITECTURAL TREATMENT (CPSRT-1-AT) -2
No.	Description	Date	10. 0
	Revisions		Designed: \$88DIV