



Kimley-Horn  
and Associates, Inc.

December 14, 2011

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23230

Mr. Paul Agnello  
Richmond District Transportation Planner  
Planning and Investment Management  
Virginia Department of Transportation  
2430 Pine Forest Drive  
Colonial Heights, VA 23834

**Re: I-95/I-64 Overlap Safety and Operations Study  
Henrico County and City of Richmond, VA  
*Task 2.3 Deliverable – Existing Conditions Calibration Report***

Dear Mr. Agnello:

Kimley-Horn and Associates, Inc. has completed the model development and calibration process for the VISSIM traffic simulation existing conditions modeling for I-95/I-64 Overlap Safety and Operations Study. Initial results and model files of the existing conditions for both AM and PM peak periods were submitted to the Study Work Group on November 23, 2011. Updated results and files for the existing conditions were submitted on December 12, 2011. The purpose of this memorandum is to document the model development and calibration process used by Kimley-Horn to match the traffic simulation model results to the traffic data collected in the field.

### ***Base Model Development***

Coding of the initial VISSIM model was completed by VDOT and submitted to Kimley-Horn. It included all network geometry, speed data, and AM and PM peak traffic signal timing. KHA then began modifications to the network to accommodate data input and output requirements and to calibrate the network to observed existing traffic conditions. The following section is a summary of the major changes made to the model from the original file created by VDOT and from software defaults.

### ***Links and Connectors***

- To facilitate freeway density data reporting, freeway links were modified to create one continuous link between each ramp.



- To create the most realistic driver behavior at ramp acceleration and deceleration areas, connectors at the end of lane drops were deleted. As a result, simulated vehicles use the lane change logic built into the driver behavior logic to merge into the adjacent lane by the end of the lane drop instead of all vehicles merging at one point at the end of the link.
- Segments with acceleration/deceleration lanes were coded as separate links that extended approximately halfway into the taper of the lane drop/add. This allows merging vehicles to utilize part of the taper section to begin or complete their lane change maneuver.

#### *Vehicle Inputs*

- Inputs were coded in 15-minute increments based on the balanced traffic volume data set developed from a combination of traffic count data and origin-destination (O-D) surveys collected for this study. A detailed description of the volume development process was described in a previous document.
- Based on a review of freeway classification counts, a standard vehicle composition of 11% heavy vehicles was applied to all vehicle inputs.

#### *Vehicle Routing Decisions*

- Vehicle routing decisions were coded in 15-minute increments based on the balanced traffic volume data set and origin-destination matrices developed by KHA. A detailed description of the volume development process was described in a previous document. All freeway routes were coded with continuous routes from entrance ramp to exit ramp. Additional surface street intersections were coded on a per intersection basis using turning movement counts. Routes through several closely spaced intersections were combined to create more realistic lane utilization behavior.

#### *Desired Speed Decisions*

- The number of speed decisions in the original model was reduced to only at those locations where posted speeds change, such as the transition from arterials to ramps and ramps to freeways.
- Additional speed distributions were created to reflect the reduced speeds on freeway-to-freeway connector ramps such as the Bryan Park interchange, and varying posted speed limits on arterials.

#### *Conflict Areas*

- As part of the revisions to the merge areas described under Links and Connectors, the conflict areas at the end of the merge sections were removed.



### *Stop Signs*

- To incorporate right-turn-on-red (RTOR) movements, the starting point of the right-turn connectors at the signalized intersections were moved upstream of the stop bar and the RTOR stop signs placed on these connectors.

### *Signalized Intersections*

- For simplicity, all traffic signals were converted to the Ring-Barrier-Controller (RBC) signal emulator in VISSIM.
- The time-of-day schedule for traffic signals in VISSIM must be coded in simulation seconds. The start time for the coordinated plan was set to 0 seconds for all signals.

### *Data Collection*

- Link Evaluation was activated for all freeway links. Segment length was set to a value greater than link length to ensure reporting of entire link length. This is used to generate link density and average throughput and speed.
- Nodes were created at all arterial study intersections. This is used to report intersection throughput, delay, and queuing.
- Data Collection points were placed to approximate the location of count stations and Wavetronix data collection in the field. These points are used to report speeds in the simulation for comparison with data collected in the field.

### **Error Checking**

The initial simulation runs were used for visual checks of the realism of the model and for minor coding errors. Examples of corrections made during this step are:

- Adding missing connectors
- Correcting placement of signal heads/stop bars
- Extending the freeway entry links further upstream approximately one mile to allow vehicles time to move to the correct lane.

### **Calibration Methodology**

The unique geometric features and design deficiencies posed many challenges to the calibration process. Short merge and weave sections in the study area force drivers to make challenging maneuvers that cannot be replicated with the default settings in VISSIM. The following list of significant changes made to the model to replicate existing conditions are identified below:

- Based on past experience and discussions with the VISSIM software manufacturer (PTV), KHA has determined that in heavy weaving sections, the Urban (motorized) driver behavior produces more realistic results. This behavior model (Wiedemann 74), which is usually used on arterial streets, results in more



lane change decisions per second than the freeway model (Wiedemann 99), which is better at replicating free-flow freeway conditions. The Urban (motorized) behavior model was initially coded on all merge/diverge areas and ultimately on all links within the overlap section and direct connector ramps.

- The corridor has several locations where multiple lanes are forced to merge into one lane to connect from one freeway to another. Traffic counts collected in these one-lane sections show that flow rates are much higher than a standard freeway lane, reaching 2300-2500 vehicles per hour. The default saturation flow rate of approximately 1900 vehicles per hour per lane was not sufficient to model this situation. To help account for these conditions, two additional categories of saturation flow rate were created. The first step up was called “high sat flow rate” and reflects approximately 2100 vehicles per hour per lane. This rate was coded for all links in advance of the ramps where the high flows are present to allow vehicles to start to congregate in the outside lanes. In the highest flow rate sections, a third category was created called “extreme high sat flow” with a flow rate of approximately 2400 vehicles per hour per lane.
- The accepted deceleration rate for lane changes for the higher flow rate behavior types was increased to make drivers more aggressive. The safety distance reduction factor was cut in half from the default value of 0.8 to 0.4 to make drivers accept smaller distances between other cars when making a lane change. This was based on observations of the simulation. The smallest value recommended by the software developer, PTV, is 0.25 for the most aggressive drivers.
- The “time before diffusion” parameter was reduced to 30 seconds in heavy weave sections (from the default value of 60 seconds). This was an attempt to balance allowing some vehicles to stop and block a lane to wait for a gap without waiting for an unreasonable time. Based on field observations, 30 seconds appeared to be a more appropriate length of time that any vehicle would be stopped before someone would let them in.
- A very powerful parameter on each connector is the “Lane Change” distance. This value specifies how many feet upstream of a connector (such as an exit ramp) that vehicles start to make their lane changes. Initial attempts were made to set these values to a standard value of ½ mile or 1 mile upstream of a ramp. However, due the unique nature of the overlap geometry and traffic flows, this proved unrealistic. For example, a large percentage of traffic within the overlap exits to I-64 in both directions. Coding the lane change distance for those ramps to a high value resulted in unrealistic congestion, because too many vehicles struggled to move to the right lane throughout the overlap. Based on several observations of simulated traffic flow, it was determined that a value of 1750 feet provided the best lane utilization within the overlap. Most ramps required many trial-and-error iterations to determine the ideal distance.



- In a few cases, changing only the Lane Change distance could not ensure that vehicles would be in the proper lane as they do in the field. This occurred primarily on the entries to the network. In these cases, short “dummy” connectors were added upstream of the overlap to force certain routes to make their lane changes further upstream. This is a common practice in VISSIM recommended by the software developer and is typically required when a vehicle’s target lanes become congested.

### ***Calibration Special Case***

As described above, the lane drop segments were revised to take advantage of the built-in lane change behavior of VISSIM instead of using a connector with at the end of the lane drop to represent a merge. This produces a more realistic representation of the behavior of an entrance ramp acceleration lane or lane drop. However, the extremely short merge distances present in this study area highlighted one of the limitations of VISSIM. If a vehicle reaches the end of the acceleration lane before it can make a lane change to merge into the next lane, it comes to a stop and waits for a gap, eventually being removed from the network if it does not find an opening. Unfortunately, VISSIM does not do a very good job replicating driver behavior in cases when most vehicles in the next lane slow down when they see a vehicle in the adjacent lane traveling at a significantly slower speed. VISSIM vehicles will typically pass by a stopped or slowed vehicle at nearly full speed, making it very difficult for merging vehicles to find a gap.

The first attempt to overcome this limitation was to use priority rules. They were coded to force vehicles on the main line to stop and let vehicle in when they reached the end of a lane drop. While this strategy worked in the majority of cases, when large trucks could not complete their merge in time, it frequently caused vehicles in both lanes to get “stuck” in a gridlock situation, apparently yielding to each other. For this reason, the majority of these priority rules were disabled.

To finally overcome this limitation, KHA developed an innovative custom strategy using the VAP scripting language in VISSIM. This script replicates real-world behavior by using a combination of detectors and desired speed decisions. In merge areas, vehicles are forced to slow down if they detect a vehicle in the adjacent lane going slow or stopped. The two sets of desired speed decisions leading into the merge area are altered dynamically based on the speed detected downstream in the adjacent lane. With the vehicles in each lane traveling at speeds much closer to each other, they are more likely to cooperate and let a merging vehicle into their lane. All vehicles are set back to freeway speeds near the end of the merge area.

Because the script is dynamic, it only slows vehicles down when necessary and allows the congestion to recover. This is much better alternative to more traditional methods of hard-coding the slow downs at bottlenecks and it will provide a more realistic picture of the impact of improving these merge areas in the future scenarios.

### ***Calibration Results***



Two hours of the AM and PM peak period models were calibrated to field collected data. For the AM peak period, it was the 7:15 – 8:15 AM and the 8:15 – 9:15 AM hours, and for the PM peak period, it was the 4:00 – 5:00 PM and the 5:00 – 6:00 PM hours. The calibration targets were based on two measures of effectiveness (MOEs) – traffic volume throughput and vehicle speeds. Because the model is microscopic in nature, an unrealistically modeled bottleneck at one point in the model will affect everything downstream because too much or too little traffic will pass through that point. The model bottlenecks were adjusted until traffic volumes that made it through the simulation network reached the levels measured during field data collection. As specified in the scope, the flows were calibrated based on the GEH statistic with the target thresholds in **Table 1**.

The GEH statistic is calculated as follows:

$$GEH = \sqrt{\frac{2(m - c)^2}{m + c}}$$

*Notes:*

*m = output traffic volumes from the model*

*c = input traffic volumes from the counts*

GEH < 5.0	Acceptable fit
5.0 <= GEH <= 10.0	Caution: possible model errors or bad data
GEH > 10.0	Unacceptable

**Table 1 | GEH Volume Calibration Thresholds**

Criteria	Acceptable Targets
GEH < 5.0	At least 85% of freeway links within the calibration area.
GEH < 5.0	All entry and exit links within the calibration area.
GEH < 5.0	All entrance and exit ramps with the calibration area.
GEH < 5.0	All intersection turning movements greater than 100 vph.
Individual flows within $\pm 400$ vph for flows exceeding 2700 vph.	At least 85% of applicable freeway links.
Sum of all link flows within the calibration area.	Within 5%.

The AM peak period calibration results based on these thresholds are presented in **Table 2**, while the PM peak period calibration results are presented in **Table 3**.

**Table 2 | AM Peak Calibration Results**

		<b>7:15 - 8:15</b>	<b>8:15 - 9:15</b>
GEH < 5.0	At least 85% of freeway links within the calibration area.	90%	100%
GEH < 5.0	All entry and exit links within the calibration area.	100%	100%
GEH < 5.0	All entrance and exit ramps with the calibration area.	88%	98%
GEH < 5.0	All intersection turning movements greater than 100 vph.	95%	92%
Individual flows within $\pm 400$ vph for flows exceeding 2700 vph.	At least 85% of applicable freeway links.	93%	100%
Sum of all link flows within the calibration area.	Within 5%.	3%	2%

**Table 3 | PM Peak Calibration Results**

		<b>4:00 - 5:00</b>	<b>5:00 - 6:00</b>
GEH < 5.0	At least 85% of freeway links within the calibration area.	93%	90%
GEH < 5.0	All entry and exit links within the calibration area.	100%	89%
GEH < 5.0	All entrance and exit ramps with the calibration area.	98%	91%
GEH < 5.0	All intersection turning movements greater than 100 vph.	92%	92%
Individual flows within $\pm 400$ vph for flows exceeding 2700 vph.	At least 85% of applicable freeway links.	100%	90%
Sum of all link flows within the calibration area.	Within 5%.	2%	2%

As shown in these tables, the models meet or nearly meet every calibration target for volumes. Detailed outputs of these results can be found in **Attachment A**.

Link speeds were also collected in the model at locations where field spot speed data was available such as continuous count stations and Wavetronix data collected for this study. A comparison of the speed results for the AM and PM calibration hours are presented in **Table 4** and **Table 5**. During the AM peak hours, simulated link speeds are within 10% of field measured speeds on about two-thirds of all locations measured, with 82% of them within 15%. In the first PM peak hour, 79% of all simulated link speeds are within 10% of field measured speed locations and 91% are within 15%. In the second hour, 58% of the spot speed locations are within 10% of field measured locations, and 70% are within 15%. Some of these discrepancies can be attributed to the unstable nature of traffic flow within the study area. It is difficult to collect a “typical” day in the area, because one small problem anywhere in the system can have a large effect on speeds both in the vicinity of the overlap and downstream of the overlap.



Table 4 | AM Peak Period Spot Speed Results

Location	7:15 - 8:15 AM			8:15 - 9:15 AM		
	Measured Speed (mph)	Simulated Speed (mph)	% Difference	Measured Speed (mph)	Simulated Speed (mph)	% Difference
I-64 EB West of Overlap	58	56.5	2.6%	43	57.9	34.6%
I-64 WB West of Overlap	59	55.2	6.5%	60	54.4	9.3%
I-95 SB North of Westbrook	54.8	52.5	4.2%	58.8	53.9	8.3%
I-95 NB North of Westbrook	59.7	56.6	5.2%	60	56.9	5.2%
I-95 SB North of I-95/I-64/I-195	54.6	53.1	2.8%	57.1	54.5	4.5%
I-95 NB North of I-95/I-64/I-195	62.5	58.0	7.2%	63.6	58.6	7.8%
Overlap NB North of North Blvd	55.4	53.8	3.0%	50.9	52.3	2.7%
Overlap SB North of North Blvd	58.3	54.7	6.1%	58.9	57.0	3.3%
Overlap NB South of Robin Hood	55.3	54.8	0.8%	56.8	55.6	2.1%
Overlap NB under Belvidere Overpass	55.8	52.0	6.9%	59.3	51.6	13.0%
Overlap SB under Belvidere Overpass	58.4	56.7	3.0%	58.6	57.2	2.4%
Overlap NB between Chamberlayne & 3rd St	58.2	42.8	26.4%	59.2	50.3	15.0%
Overlap SB between Chamberlayne & 3rd St	62.9	56.3	10.5%	62.8	56.8	9.6%
I-95 NB under 7th St Overpass	57.4	50.4	12.2%	56.7	55.1	2.8%
I-95 SB under 7th St Overpass	60.2	58.3	3.1%	59.9	58.4	2.4%
I-64 EB East of Overlap	63.3	58.7	7.3%	63	58.8	6.6%
I-64 WB East of Overlap	49.9	47.5	4.9%	60.4	38.7	36.0%
I-95 NB under Broad St Overpass	55.5	35.1	36.7%	55.4	48.8	11.9%
I-95 SB under Broad St Overpass	54.9	41.8	23.9%	55.2	44.8	18.9%
I-95 SB to I-195 SB	47.7	52.5	10.0%	48.1	53.4	11.0%
I-95 SB to I-64 WB	53.1	51.7	2.6%	53.2	52.0	2.3%
I-64 EB to I-195 SB	56.2	55.2	1.7%	55.2	54.9	0.6%
I-195 NB to I-64 WB	52.7	39.9	24.2%	55.8	20.6	63.1%
I-195 NB to I-95 SB	33	39.8	20.6%	36.1	41.3	14.3%
I-195 NB to I-95 NB	48.6	43.1	11.4%	50.3	46.4	7.8%
I-95 NB to I-195 SB	55.8	56.1	0.5%	54.1	56.1	3.7%
I-95 NB to I-64 WB	54.6	55.6	1.8%	54.1	55.4	2.4%
Laburnum Ave to I-64 WB	52.6	55.9	6.4%	52.1	56.2	7.8%
I-64 EB to I-95 NB	56.5	56.2	0.6%	52.3	56.2	7.4%
I-64 EB to I-95 SB	58.8	51.3	12.8%	44.5	55.5	24.7%
I-95 SB to I-64 EB	57.7	55.7	3.5%	58.4	55.8	4.4%
I-64 WB to I-95 SB/5th St	48.2	55.0	14.1%	52.8	54.1	2.4%
I-64 WB to I-95 NB	51.8	41.1	20.6%	57.1	32.2	43.7%
SUMMARY - WITHIN 10%			64%			67%
SUMMARY - WITHIN 15%			82%			82%



Table 5 | PM Peak Period Spot Speed Results

Location	4:00 - 5:00 PM			5:00 - 6:00 PM		
	Measured Speed (mph)	Simulated Speed (mph)	% Difference	Measured Speed (mph)	Simulated Speed (mph)	% Difference
I-64 EB West of Overlap	59.3	56.9	4.0%	58.1	51.0	12.2%
I-64 WB West of Overlap	60.6	56.8	6.3%	59.7	56.4	5.6%
I-95 SB North of Westbrook	54.8	60.1	9.7%	29.9	56.9	90.2%
I-95 NB North of Westbrook	59.7	58.9	1.3%	57.1	54.6	4.5%
I-95 SB North of I-95/I-64/I-195	59.2	56.9	3.9%	31.3	55.2	76.4%
I-95 NB North of I-95/I-64/I-195	63.1	57.3	9.2%	60.5	57.0	5.7%
Overlap NB North of North Blvd	58.5	53.2	9.1%	58.3	49.3	15.4%
Overlap SB North of North Blvd	56.8	57.8	1.8%	56.9	57.5	1.1%
Overlap NB South of Robin Hood	54.9	53.3	3.0%	54.3	50.7	6.7%
Overlap NB under Belvidere Overpass	53.6	42.3	21.1%	35.7	37.1	4.0%
Overlap SB under Belvidere Overpass	56.8	57.1	0.6%	58.8	52.8	10.1%
Overlap NB between Chamberlayne & 3rd St	57.5	51.8	9.9%	31.4	50.8	61.8%
Overlap SB between Chamberlayne & 3rd St	62.6	55.6	11.1%	62.1	52.7	15.1%
I-95 NB under 7th St Overpass	54.7	49.0	10.4%	53.3	48.0	10.0%
I-95 SB under 7th St Overpass	60.9	58.5	3.9%	60.9	58.2	4.5%
I-64 EB East of Overlap	61.2	56.2	8.2%	61.6	56.2	8.7%
I-64 WB East of Overlap	61.5	57.8	6.0%	61.1	57.7	5.6%
I-95 NB under Broad St Overpass	51.7	54.5	5.4%	42	56.1	33.6%
I-95 SB under Broad St Overpass	54.8	38.2	30.2%	54.3	35.8	34.1%
I-95 SB to I-195 SB	48	53.5	11.4%	46.1	50.9	10.3%
I-95 SB to I-64 WB	53.6	54.6	1.9%	52.2	53.9	3.3%
I-64 EB to I-195 SB	55.6	55.0	1.0%	54.9	35.3	35.8%
I-195 NB to I-64 WB	55.9	55.4	0.9%	53.5	51.3	4.1%
I-195 NB to I-95 SB	35.5	41.9	18.0%	32.6	21.4	34.3%
I-195 NB to I-95 NB	50.1	49.1	2.0%	28.6	13.2	53.7%
I-95 NB to I-195 SB	53.8	56.1	4.2%	54.8	56.2	2.6%
I-95 NB to I-64 WB	52.9	55.2	4.3%	52.8	55.1	4.4%
Laburnum Ave to I-64 WB	51.6	56.7	9.9%	52.1	56.6	8.7%
I-64 EB to I-95 NB	56.1	55.0	2.0%	48.5	52.5	8.2%
I-64 EB to I-95 SB	60.4	53.9	10.8%	54.4	53.1	2.3%
I-95 SB to I-64 EB	53.1	53.8	1.4%	52.4	53.9	2.9%
I-64 WB to I-95 SB/5th St	52.9	57.3	8.4%	53.2	57.4	7.9%
I-64 WB to I-95 NB	57.1	55.0	3.7%	52.4	55.2	5.4%
SUMMARY - WITHIN 10%				79%		58%
SUMMARY - WITHIN 15%				91%		70%



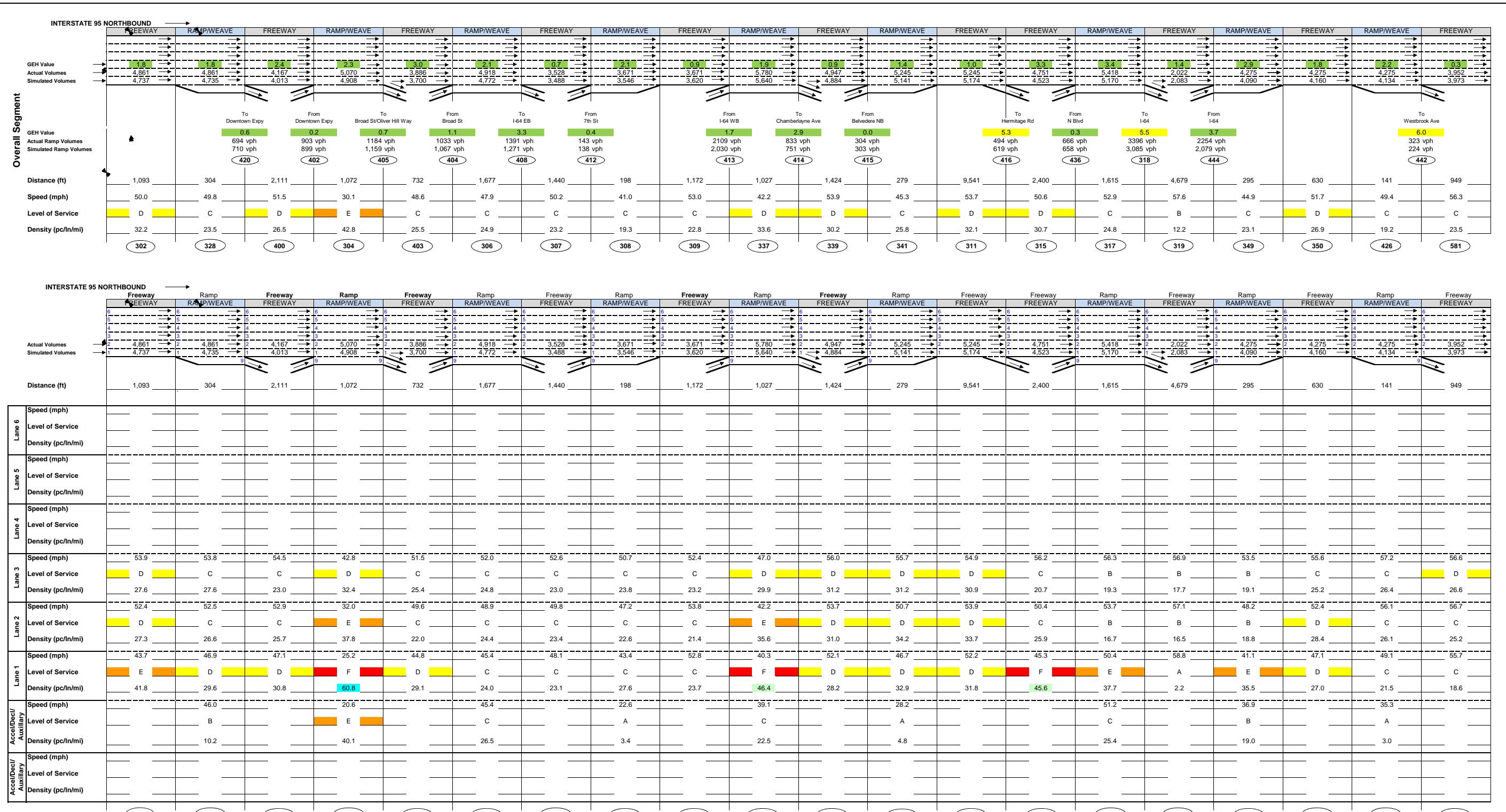
## **Summary**

The unique geometry and traffic patterns of the I-95/64 Overlap study area posed some challenges for microsimulation modeling. The short merge/weave sections and numerous lane drops required several modifications to the default parameters in VISSIM and required the use of an add-on custom logic script developed by KHA to more accurately replicate the congestion behavior in the corridor. Saturation flow rates on some segments of the corridor approach the limits of the simulation software.

The goal of calibrating the models to existing conditions is to replicate a “typical” weekday, but the likelihood of collecting traffic and speed data throughout an entire peak period in this area without an incident or other non-typical slowdown is very low.

Because the field-collected traffic volume and speed data set included some of these events, it may not be feasible to meet every calibration target. However, the vast majority of these targets have been met. The model is a valid representation of study area traffic conditions and can be used to compare the relative impacts of proposed improvement alternatives.

DRAFT

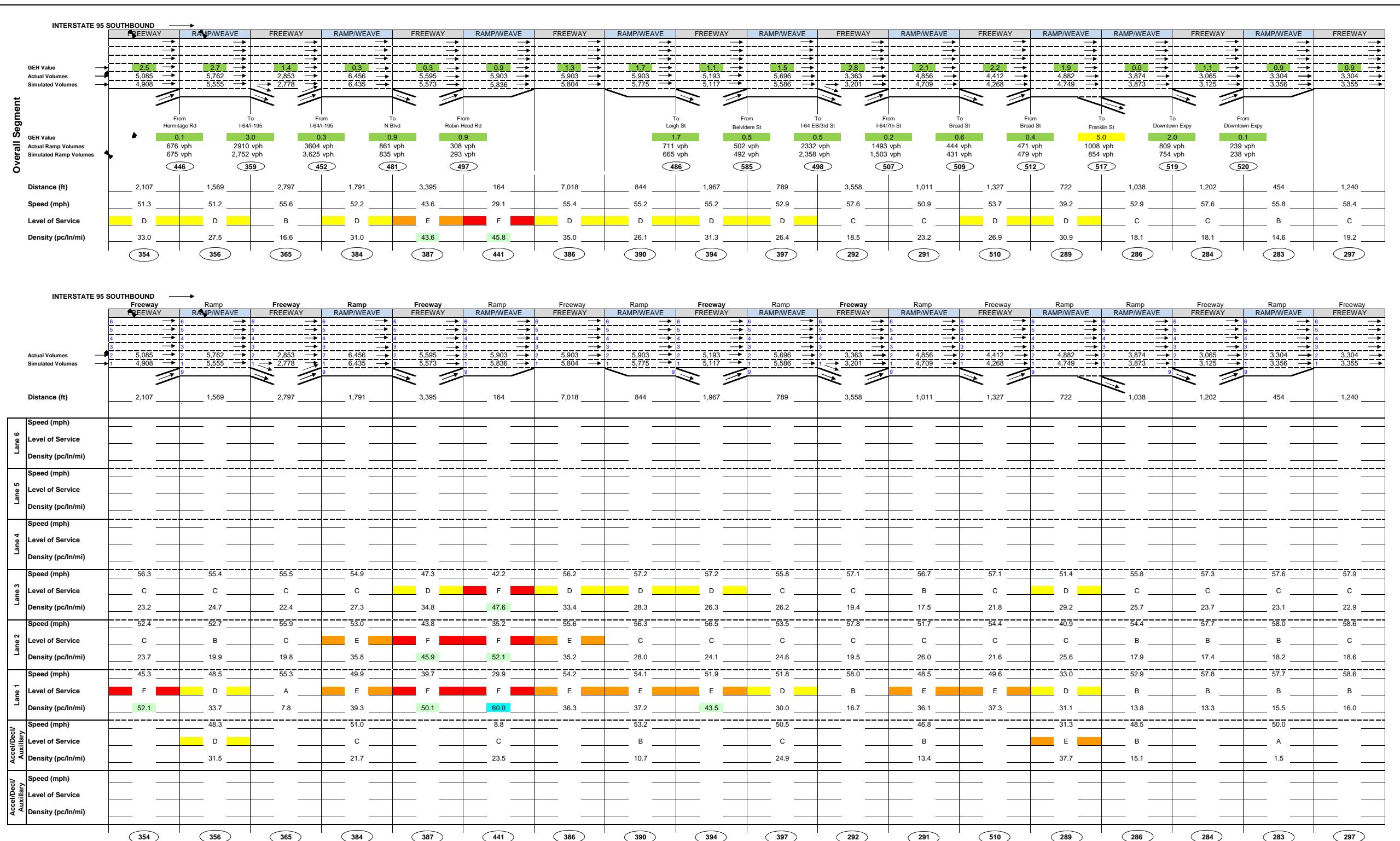


#### LEGEND

500	Link Number
Freeway Density (pc/in/mi)	Weave/Ramp Density (pc/in/mi)
LOS F: 45 and above	LOS F: 43 and above
LOS E: 35 to 45	LOS E: 35 to 43
LOS D: 26 to 35	LOS D: 28 to 35
LOS C: 18 to 26	LOS C: 20 to 28
LOS B: 11 to 18	LOS B: 10 to 20
LOS A: 0 to 11	LOS A: 0 to 10

Freeway, Weave, and Ramp Density Coloring  
Degrees of LOS F:

- Density above 75 pc/in/mi
- Density above 55 pc/in/mi
- Density above 43 pc/in/mi



#### LEGEND

500 Link Number

Freeway Density (pc/in/mi)  
LOS F 45 and above  
LOS E 35 to 45  
LOS D 26 to 35  
LOS C 18 to 26  
LOS B 11 to 18  
LOS A 0 to 11

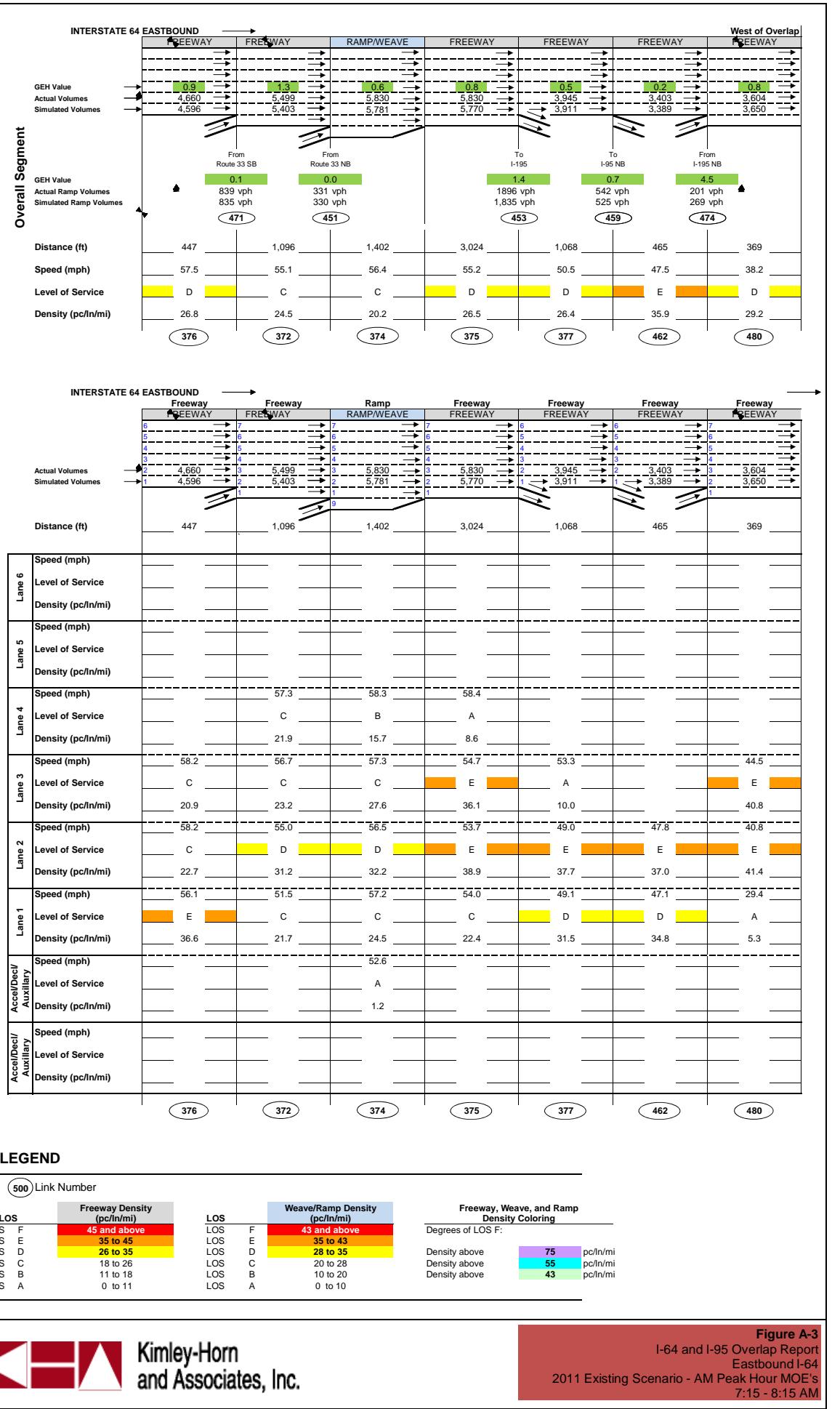
Weave/Ramp Density (pc/in/mi)  
LOS F 43 and above  
LOS E 35 to 43  
LOS D 28 to 35  
LOS C 20 to 28  
LOS B 10 to 20  
LOS A 0 to 10

Freeway, Weave, and Ramp Density Coloring  
Degrees of LOS F:  
Density above 75 pc/in/mi  
Density above 55 pc/in/mi  
Density above 43 pc/in/mi



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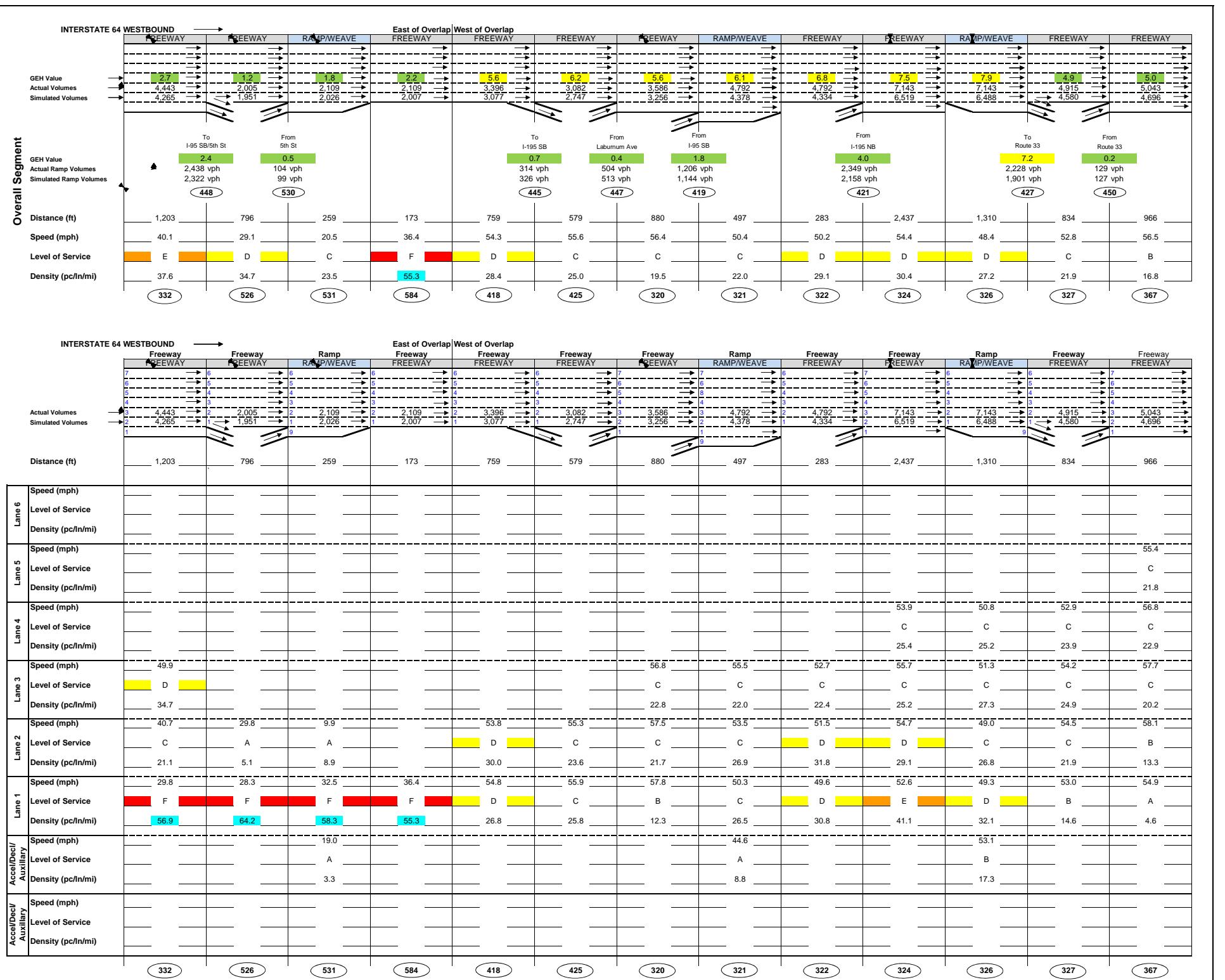
Figure A-2  
I-64 and I-95 Overlap Report  
Southbound I-95  
2011 Existing Scenario - AM Peak Hour MOE's  
7:15 - 8:15 AM



**Figure A-3**  
I-64 and I-95 Overlap Report  
Eastbound I-64  
2011 Existing Scenario - AM Peak Hour MOE's  
7:15 - 8:15 AM



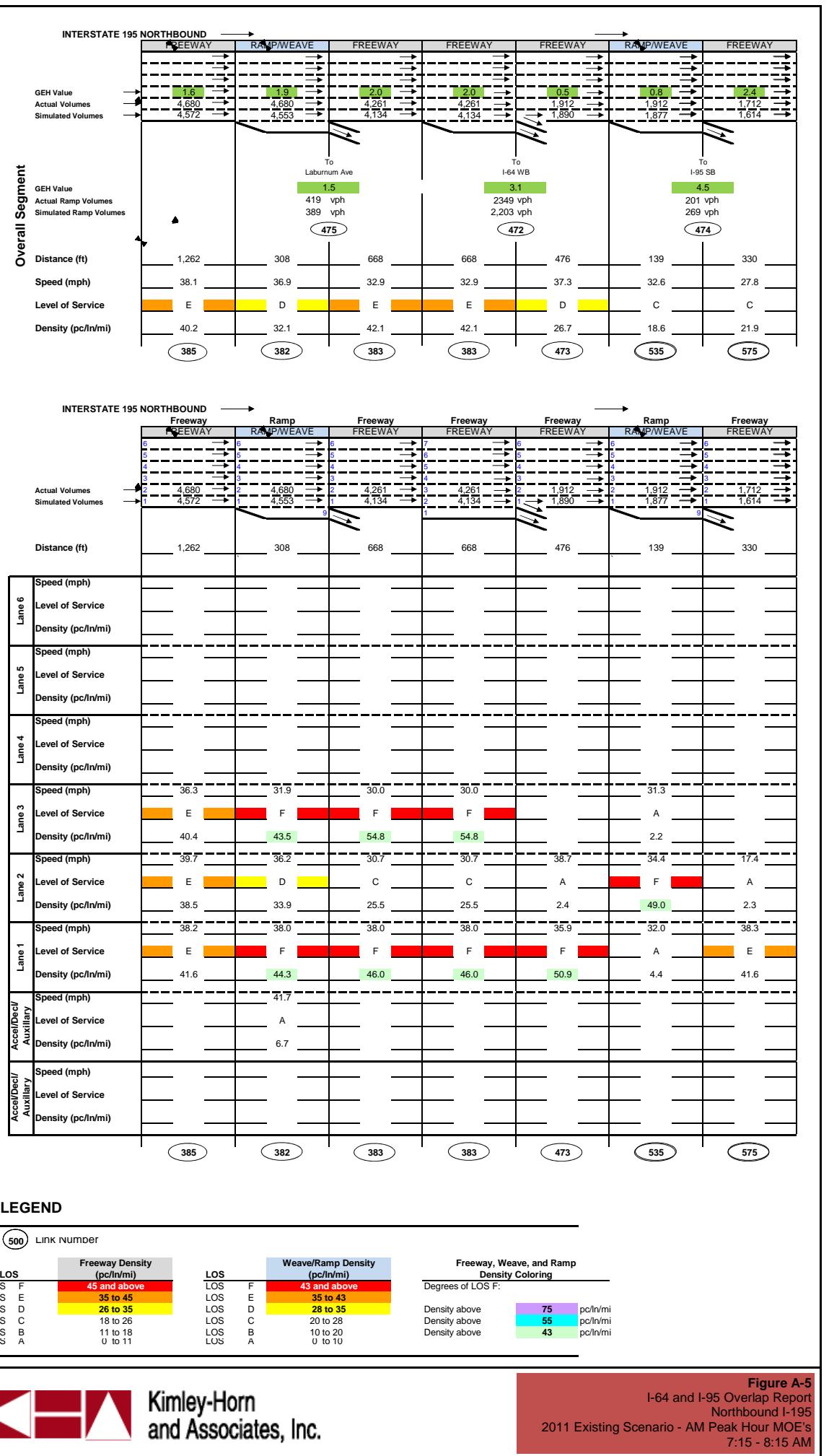
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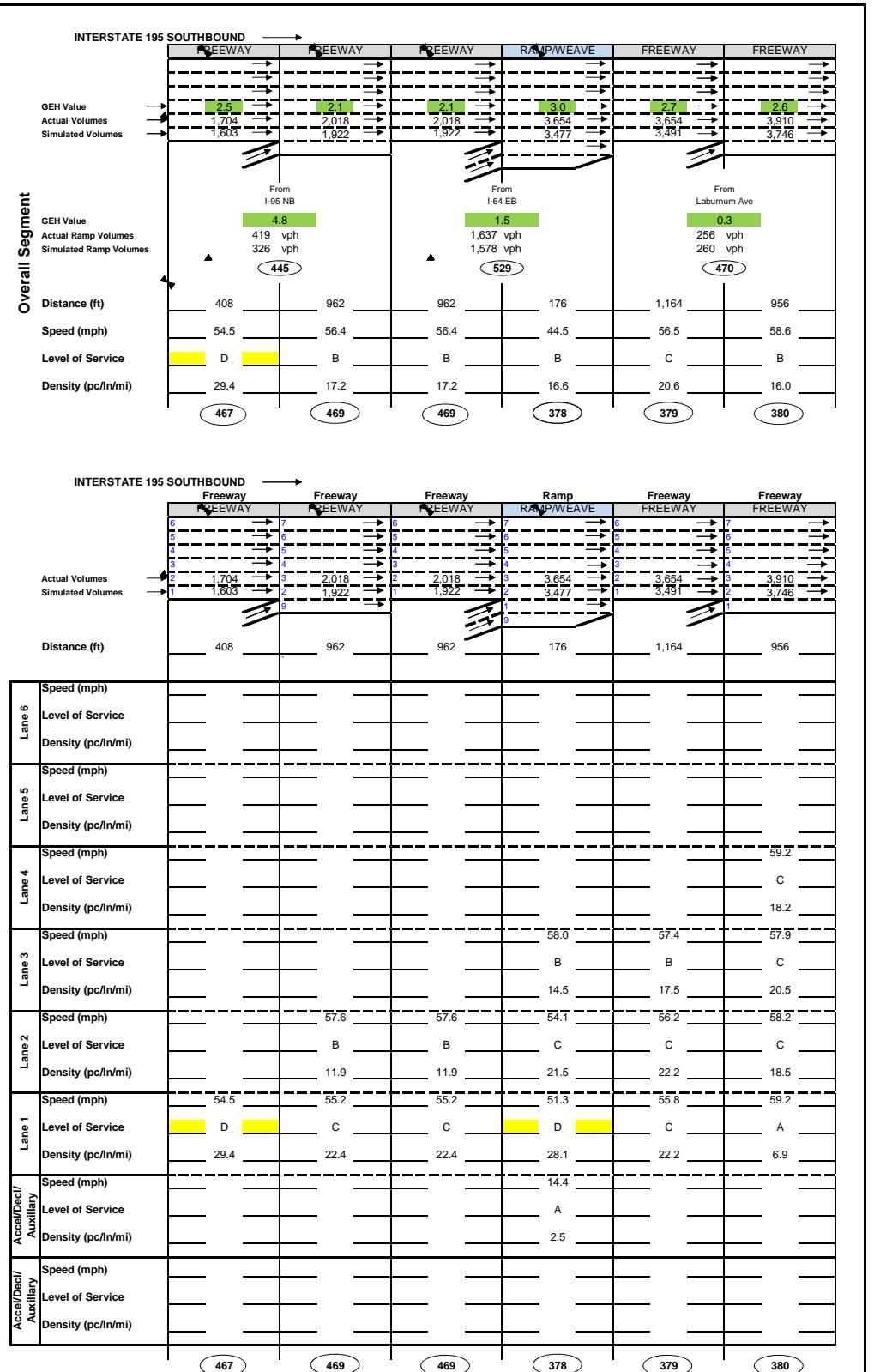
#### LEGEND

500 Link Number

Freeway Density (pc/in/mi)		Weave/Ramp Density (pc/in/mi)		Freeway, Weave, and Ramp Density Coloring	
LOS F	45 and above	LOS F	43 and above	Degrees of LOS F:	
LOS E	35 to 45	LOS E	35 to 43	Density above 75 pc/in/mi	
LOS D	26 to 35	LOS D	28 to 35	Density above 55 pc/in/mi	
LOS C	18 to 26	LOS C	20 to 28	Density above 43 pc/in/mi	
LOS B	11 to 18	LOS B	10 to 20		
LOS A	0 to 11	LOS A	0 to 10		



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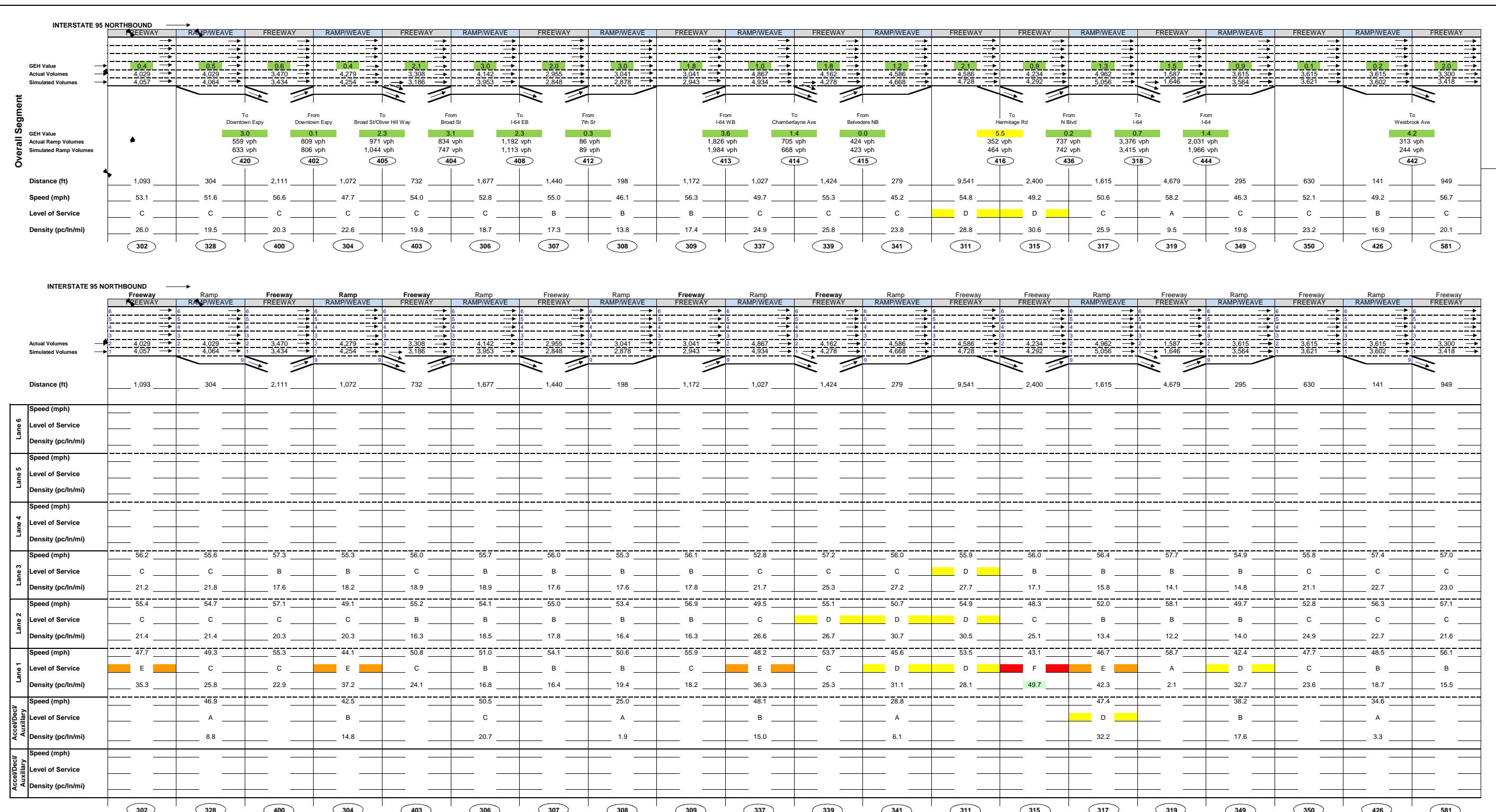
#### LEGEND

500 LINK NUMBER

LOS	Freeway Density (pc/lv/mi)	
	F	E
LOS F	45 and above	43 and above
LOS E	35 to 45	35 to 43
LOS D	26 to 35	28 to 35
LOS C	18 to 26	20 to 28
LOS B	11 to 18	10 to 20
LOS A	0 to 11	0 to 10

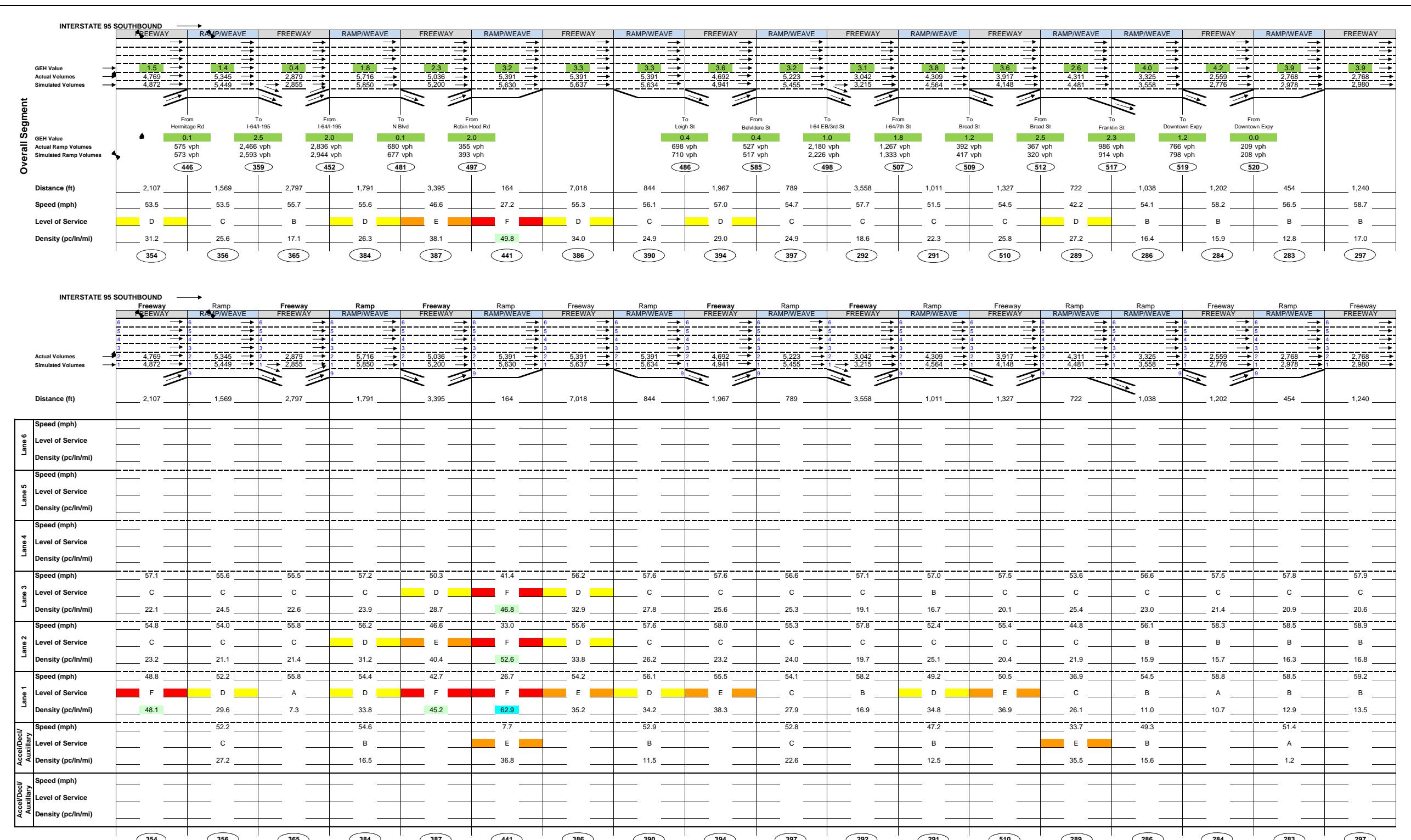
LOS	Weave/Ramp Density (pc/lv/mi)	
	F	E
LOS F	43 and above	43 and above
LOS E	35 to 43	35 to 43
LOS D	28 to 35	28 to 35
LOS C	20 to 28	20 to 28
LOS B	10 to 20	10 to 20
LOS A	0 to 10	0 to 10

Freeway, Weave, and Ramp Density Coloring		
Degrees of LOS F:		
Density above	75	pc/lv/mi
Density above	55	pc/lv/mi
Density above	43	pc/lv/mi



#### LEGEND

500	Link Number
LOS	Freeway Density (pc/in/mi)
F	45 and above
E	35 to 45
D	26 to 35
C	18 to 26
B	11 to 18
A	0 to 11
LOS	Ramp/Weave Density (pc/in/mi)
F	43 and above
E	35 to 43
D	28 to 35
C	20 to 28
B	10 to 20
A	0 to 10
Degrees of LOS F:	
Density above 75	pc/in/mi
Density above 55	pc/in/mi
Density above 43	pc/in/mi



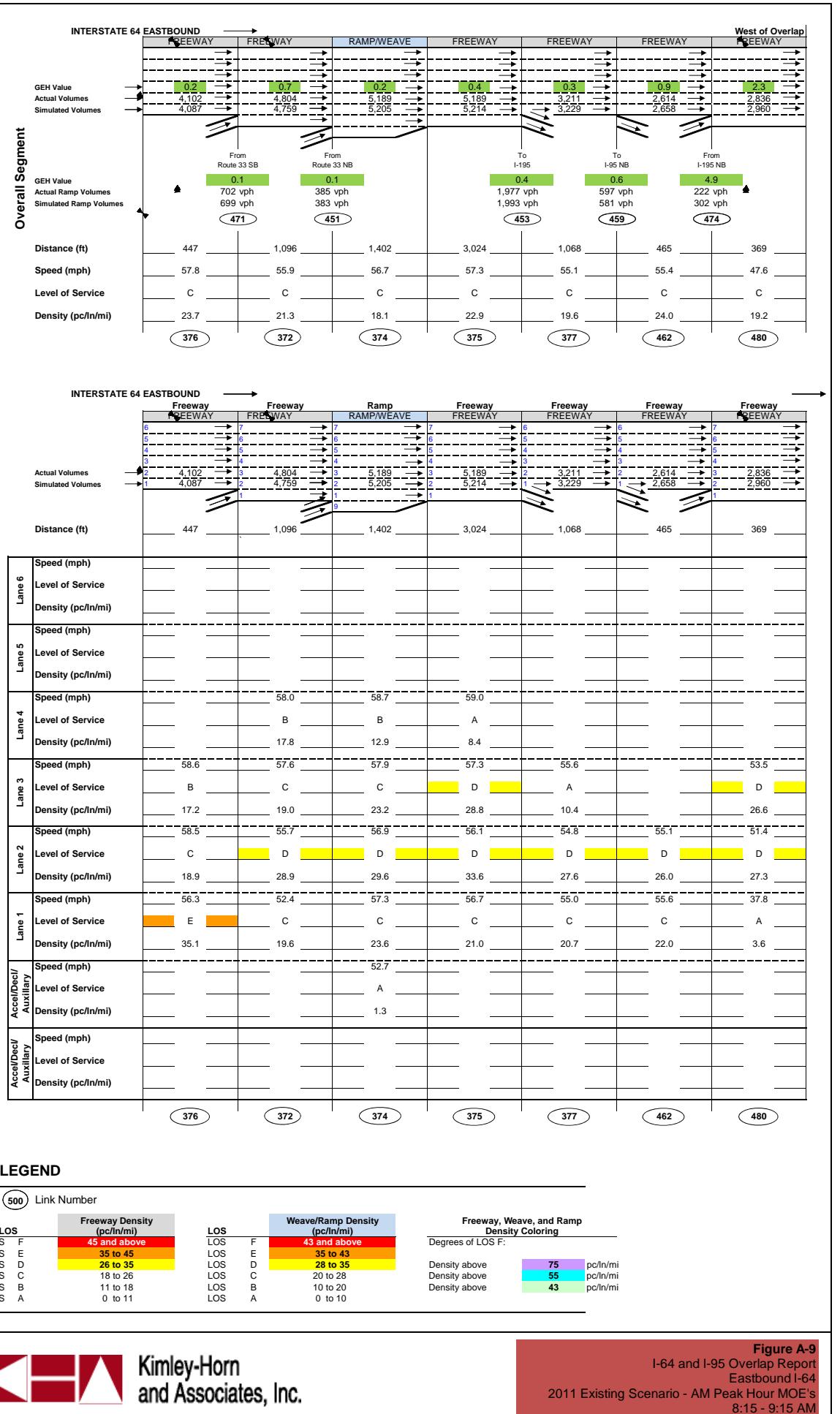
#### LEGEND

500 Link Number

Freeway Density (pc/in/mi)	LOS F	Freeway, Weave, and Ramp Density (pc/in/mi)	LOS F	Degrees of LOS F:
45 and above	LOS F	43 and above	LOS F	Density above 75 pc/in/mi
35 to 45	LOS E	35 to 43	LOS E	Density above 55 pc/in/mi
26 to 35	LOS D	28 to 35	LOS D	Density above 43 pc/in/mi
18 to 26	LOS C	20 to 28	LOS C	
11 to 18	LOS B	10 to 20	LOS B	
0 to 11	LOS A	0 to 10	LOS A	

Figure A-8

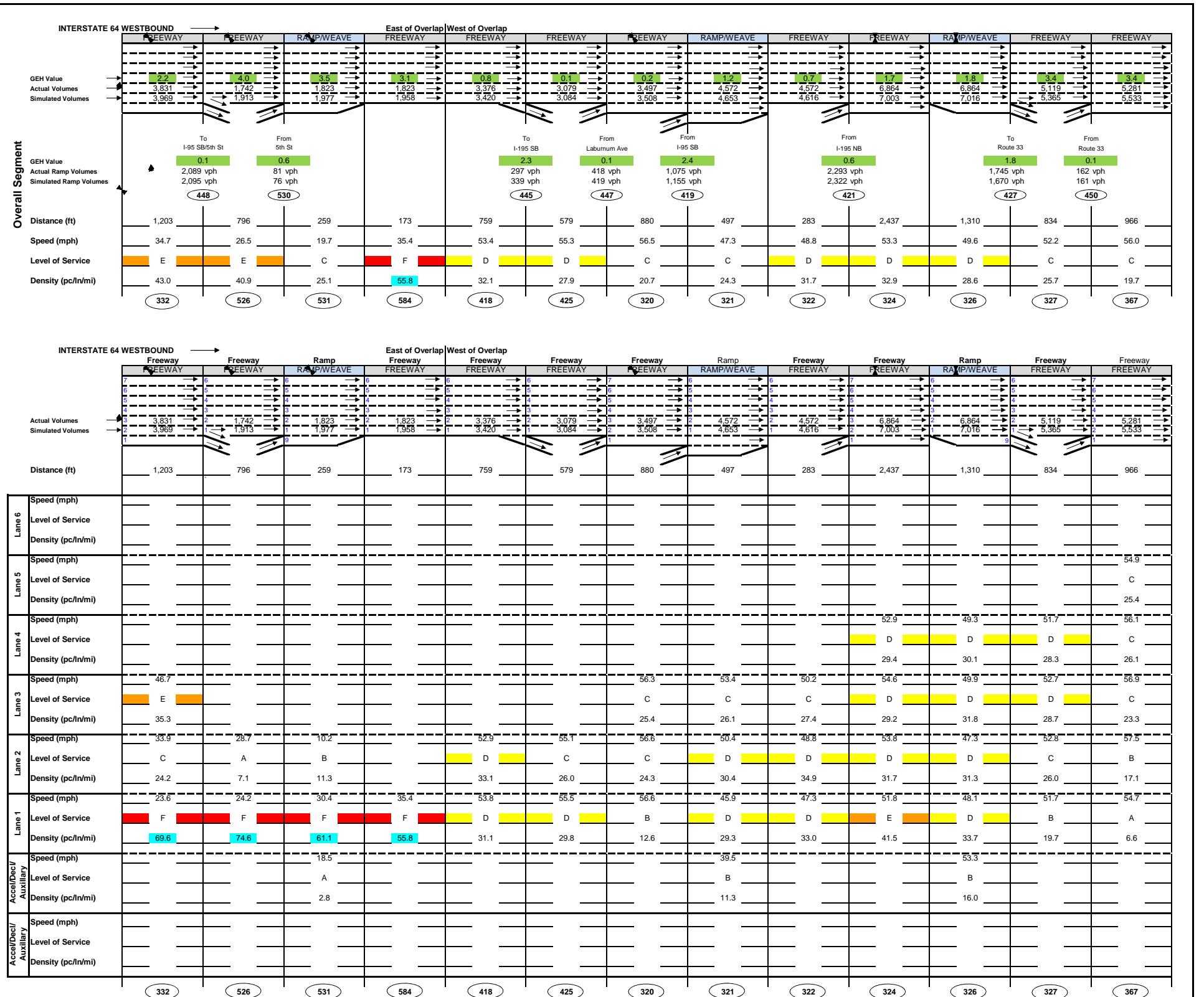
I-64 and I-95 Overlap Report  
Southbound I-95  
2011 Existing Scenario - AM Peak Hour MOE's  
8:15 - 9:15 AM



**Figure A-9**  
I-64 and I-95 Overlap Report  
Eastbound I-64  
2011 Existing Scenario - AM Peak Hour MOE's  
8:15 - 9:15 AM



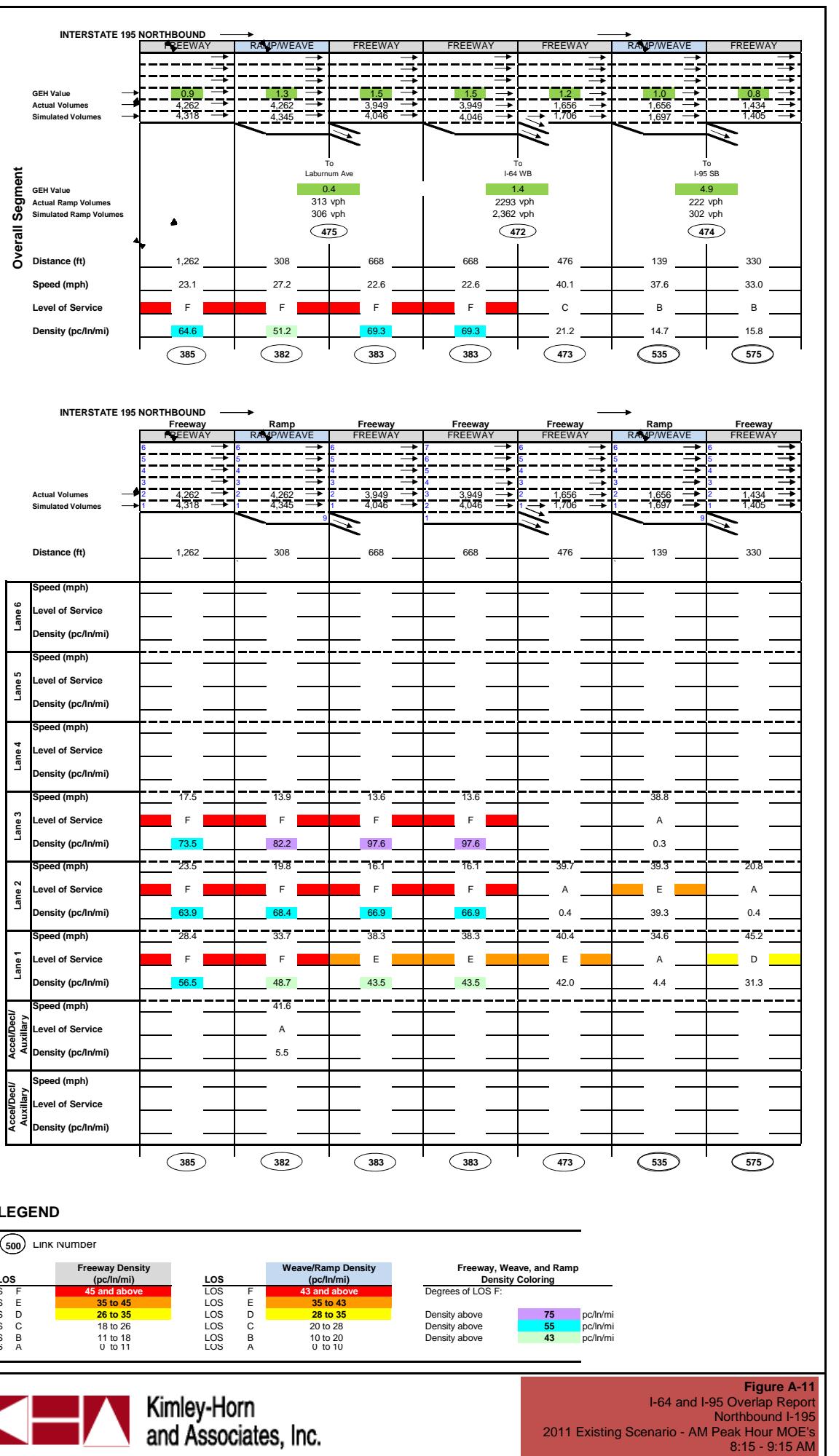
Kimley-Horn  
and Associates, Inc.



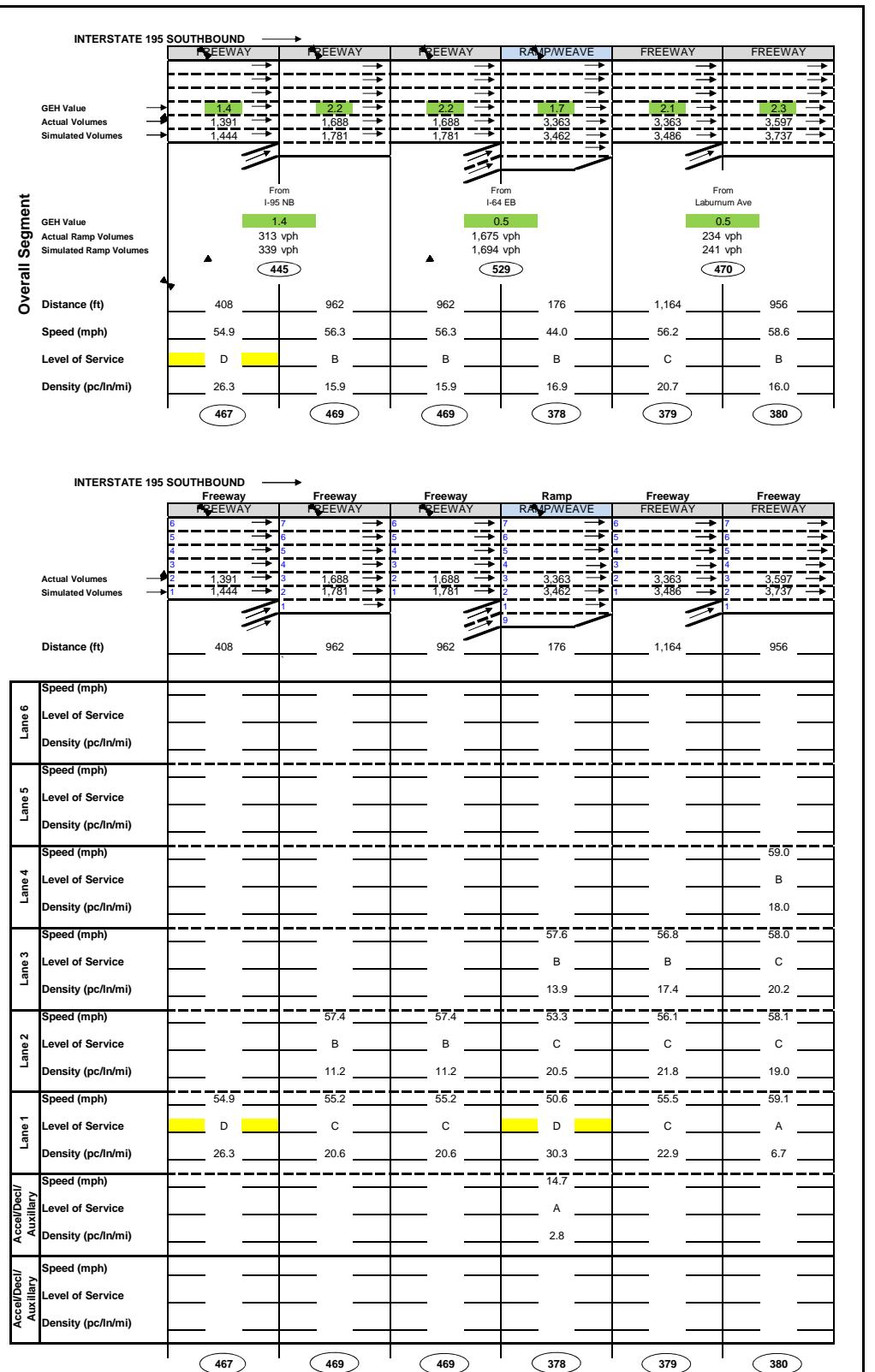
#### LEGEND

500	Link Number
LOS	Freeway Density (pc/in/mi)
LOS F	45 and above
LOS E	35 to 45
LOS D	26 to 35
LOS C	18 to 26
LOS B	11 to 18
LOS A	0 to 11
LOS	Weave/Ramp Density (pc/in/mi)
LOS F	43 and above
LOS E	35 to 43
LOS D	28 to 35
LOS C	20 to 28
LOS B	10 to 20
LOS A	0 to 10
	Freeway, Weave, and Ramp Density Coloring
	Degrees of LOS F:
	Density above 75 pc/in/mi
	Density above 55 pc/in/mi
	Density above 43 pc/in/mi

**Figure A-10**  
I-64 and I-95 Overlap Report  
Westbound I-64  
2011 Existing Scenario - AM Peak Hour MOE's  
8:15 - 9:15 AM



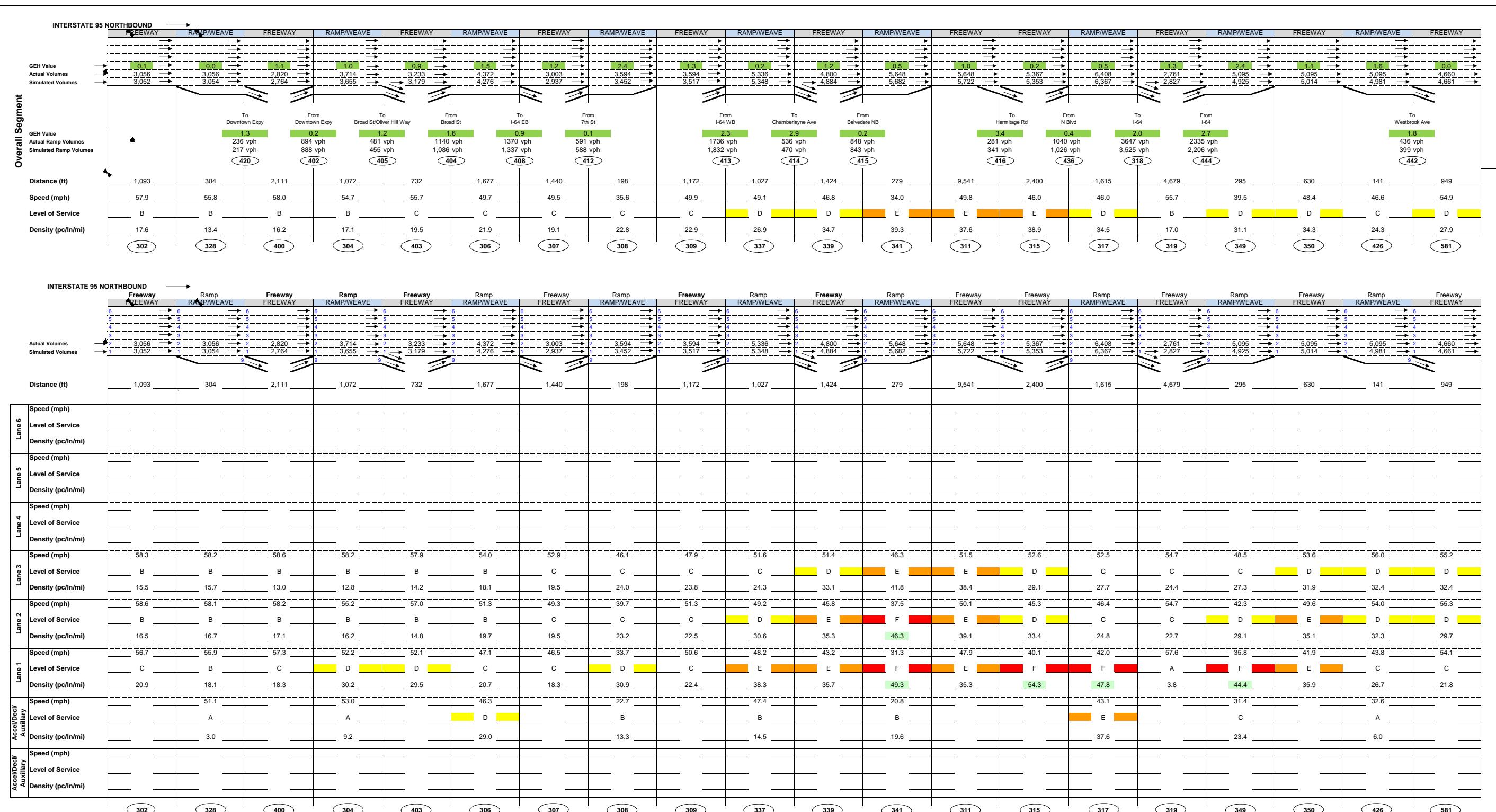
Kimley-Horn  
and Associates, Inc.



#### LEGEND

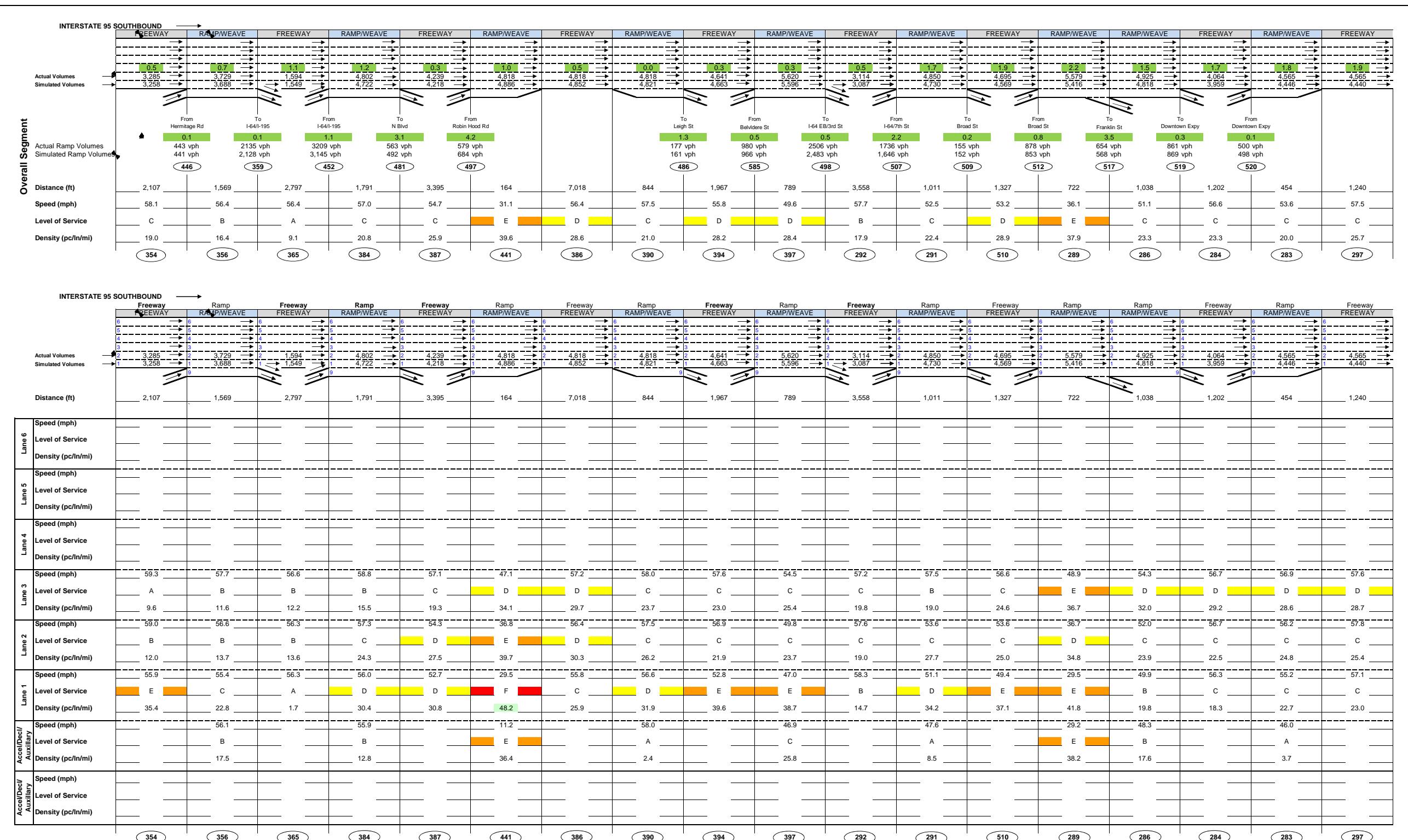
500 LINK NUMBER

LOS	Freeway Density (pc/lv/mi)		Weave/Ramp Density (pc/lv/mi)		Freeway, Weave, and Ramp Density Coloring	
	F	E	F	E	Degrees of LOS F:	
LOS F	45 and above		43 and above			
LOS E	35 to 45		35 to 43			
LOS D	26 to 35		28 to 35			
LOS C	18 to 26		20 to 28			
LOS B	11 to 18		10 to 20			
LOS A	0 to 11		0 to 10			



#### LEGEND

500	Link Number
Freeway Density (pc/in/mi)	Weave/Ramp Density (pc/in/mi)
LOS F 45 and above	LOS F 43 and above
LOS E 35 to 45	LOS E 35 to 43
LOS D 26 to 35	LOS D 28 to 35
LOS C 18 to 26	LOS C 20 to 28
LOS B 11 to 18	LOS B 10 to 20
LOS A 0 to 11	LOS A 0 to 10
Degrees of LOS F:	
Density above 75 pc/in/mi	
Density above 55 pc/in/mi	
Density above 43 pc/in/mi	



#### LEGEND

500 Link Number

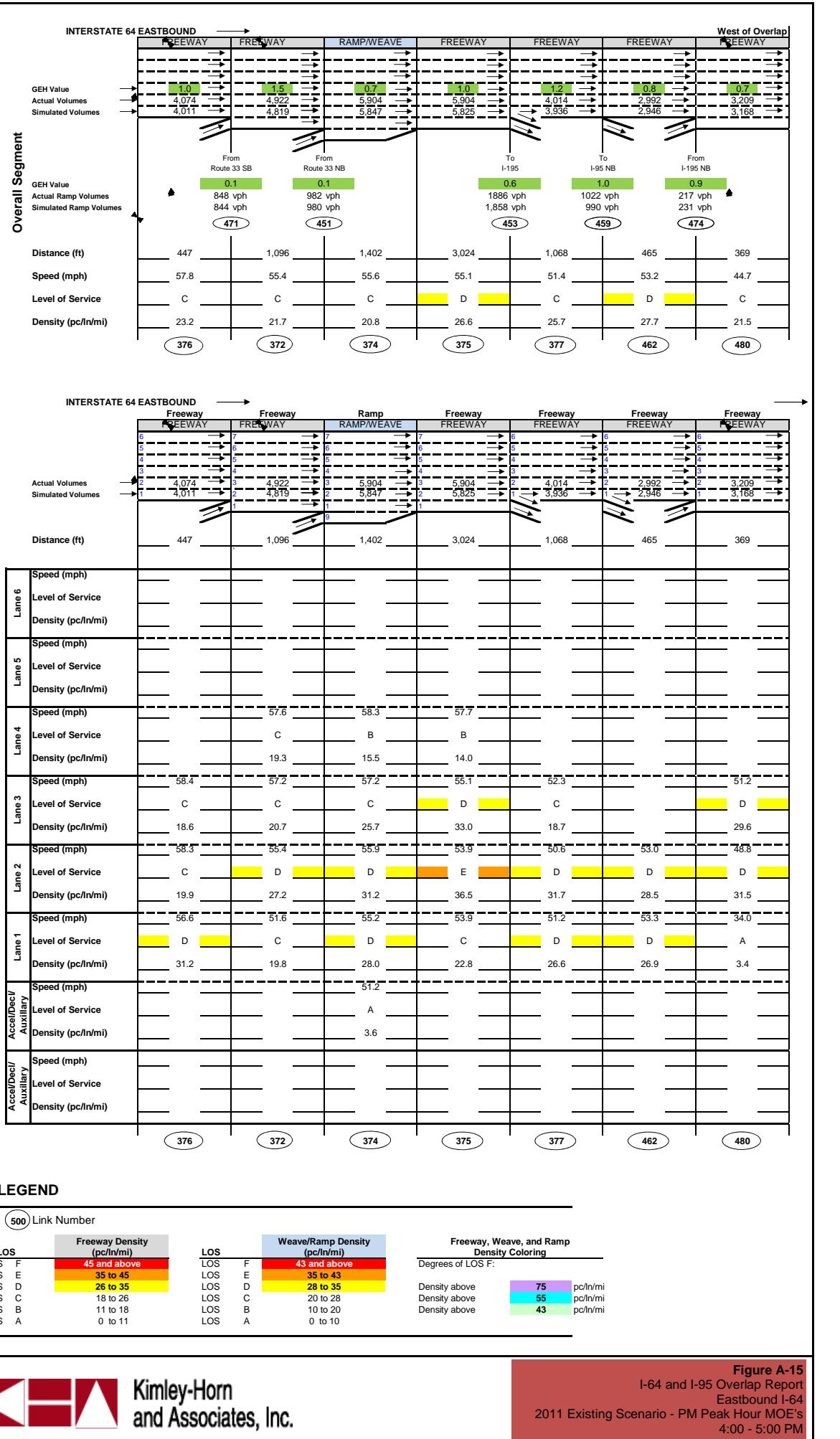
Freeway Density (pc/in/mi)		Weave/Ramp Density (pc/in/mi)		Freeway, Weave, and Ramp Density Coloring	
LOS F	45 and above	LOS F	43 and above	Degrees of LOS F:	
LOS E	35 to 45	LOS E	35 to 43	Density above 75 pc/in/mi	
LOS D	26 to 35	LOS D	28 to 35	Density above 55 pc/in/mi	
LOS C	18 to 26	LOS C	20 to 28	Density above 43 pc/in/mi	
LOS B	11 to 18	LOS B	10 to 20		
LOS A	0 to 11	LOS A	0 to 10		

Figure A-14

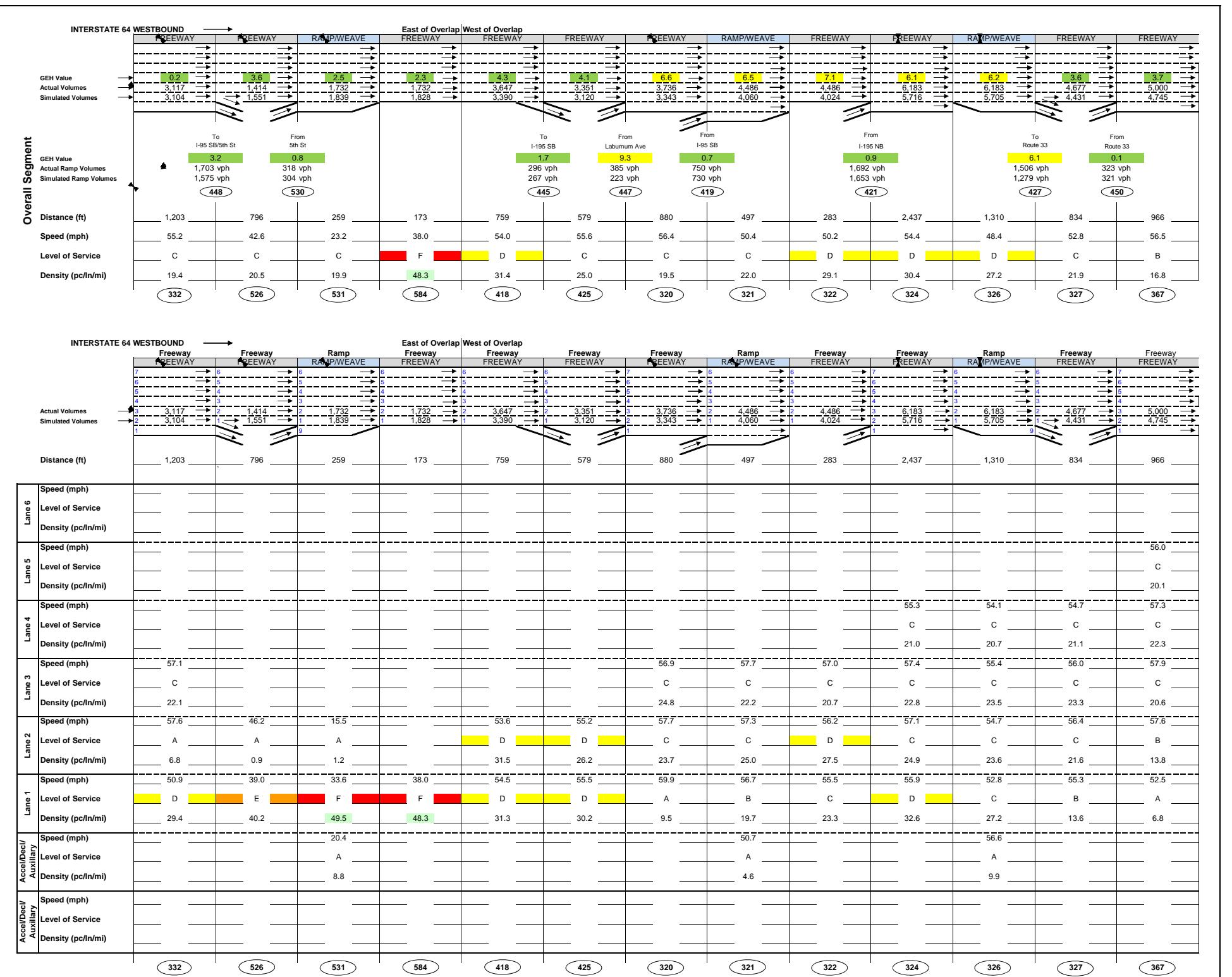
I-64 and I-95 Overlap Report  
Southbound I-95  
2011 Existing Scenario - PM Peak Hour MOE's  
4:00 - 5:00 PM



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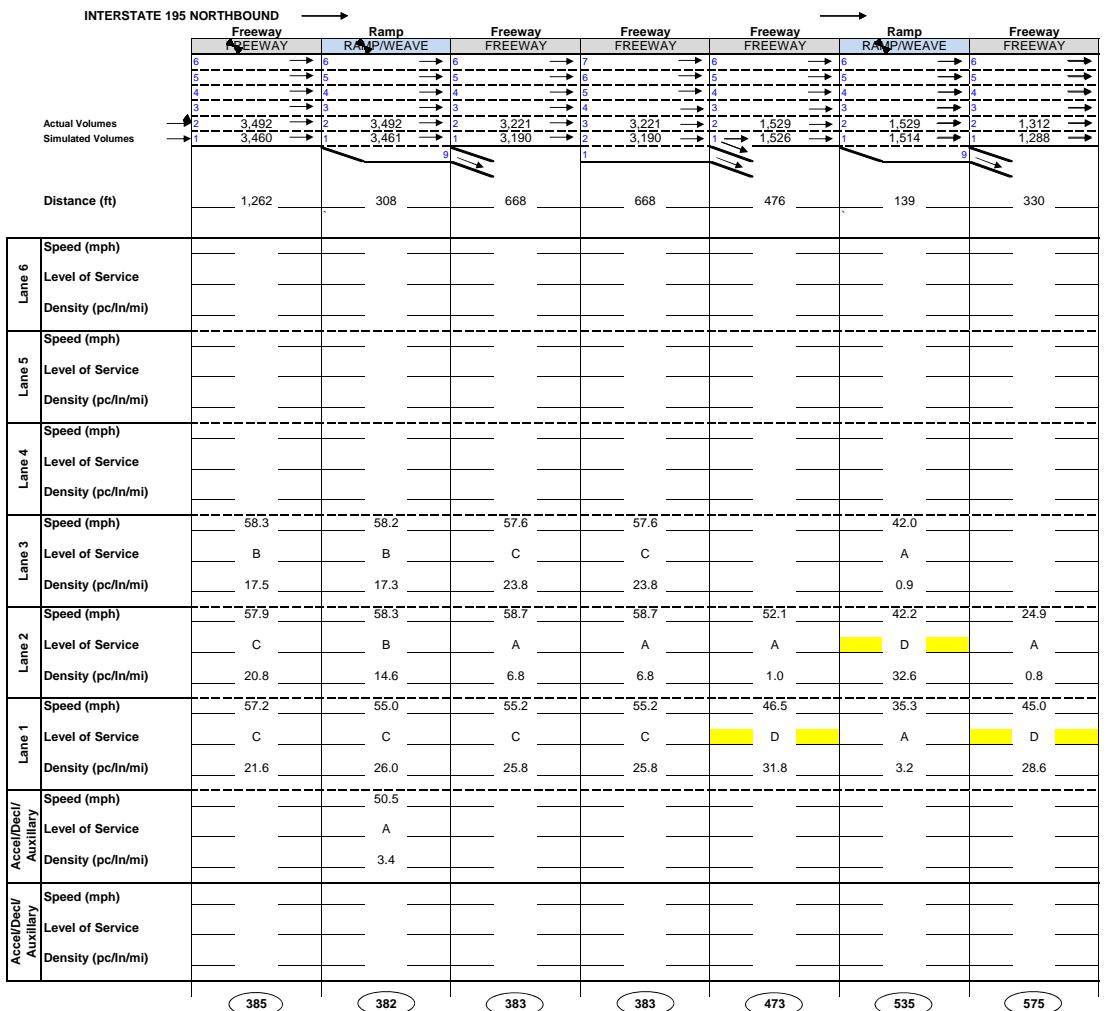
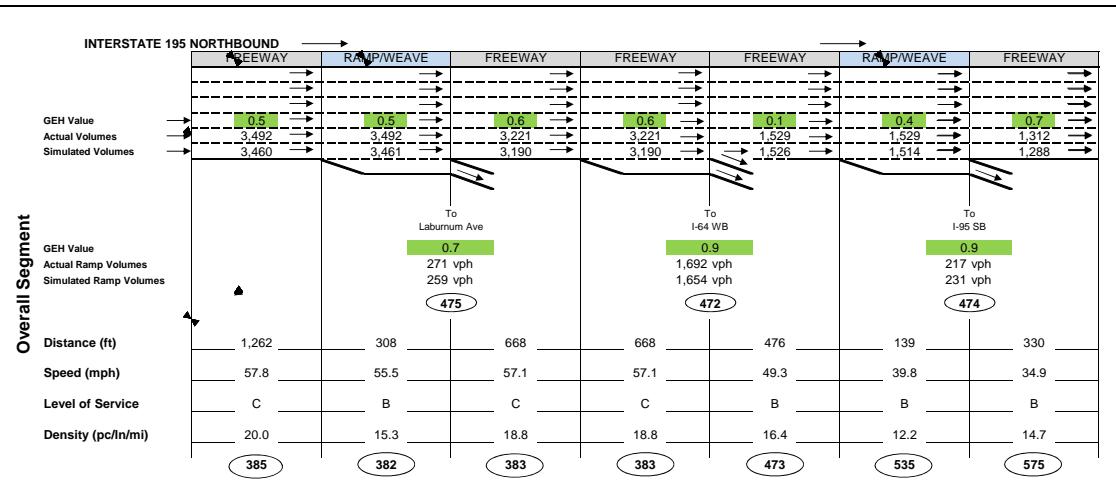
#### LEGEND

500 Link Number

LOS	Freeway Density (pc/in/mi)		Weave/Ramp Density (pc/in/mi)		Freeway, Weave, and Ramp Density Coloring	
	F	45 and above	F	43 and above	E	35 to 45
LOS E						
LOS D						
LOS C						
LOS B						
LOS A						

Degrees of LOS F:

- Density above 75 pc/in/mi
- Density above 55 pc/in/mi
- Density above 43 pc/in/mi



#### LEGEND

500 Link Number

LOS	Freeway Density (pc/in/mi)		Weave/Ramp Density (pc/in/mi)		Freeway, Weave, and Ramp Density Coloring		Weave/Ramp Density (pc/in/mi)	
	F	E	F	E	D	C	B	A
LOS F	45 and above		43 and above					
LOS E	35 to 45		35 to 43					
LOS D	26 to 35		28 to 35					
LOS C	18 to 26		20 to 28					
LOS B	11 to 18		10 to 20					
LOS A	0 to 11		0 to 10					

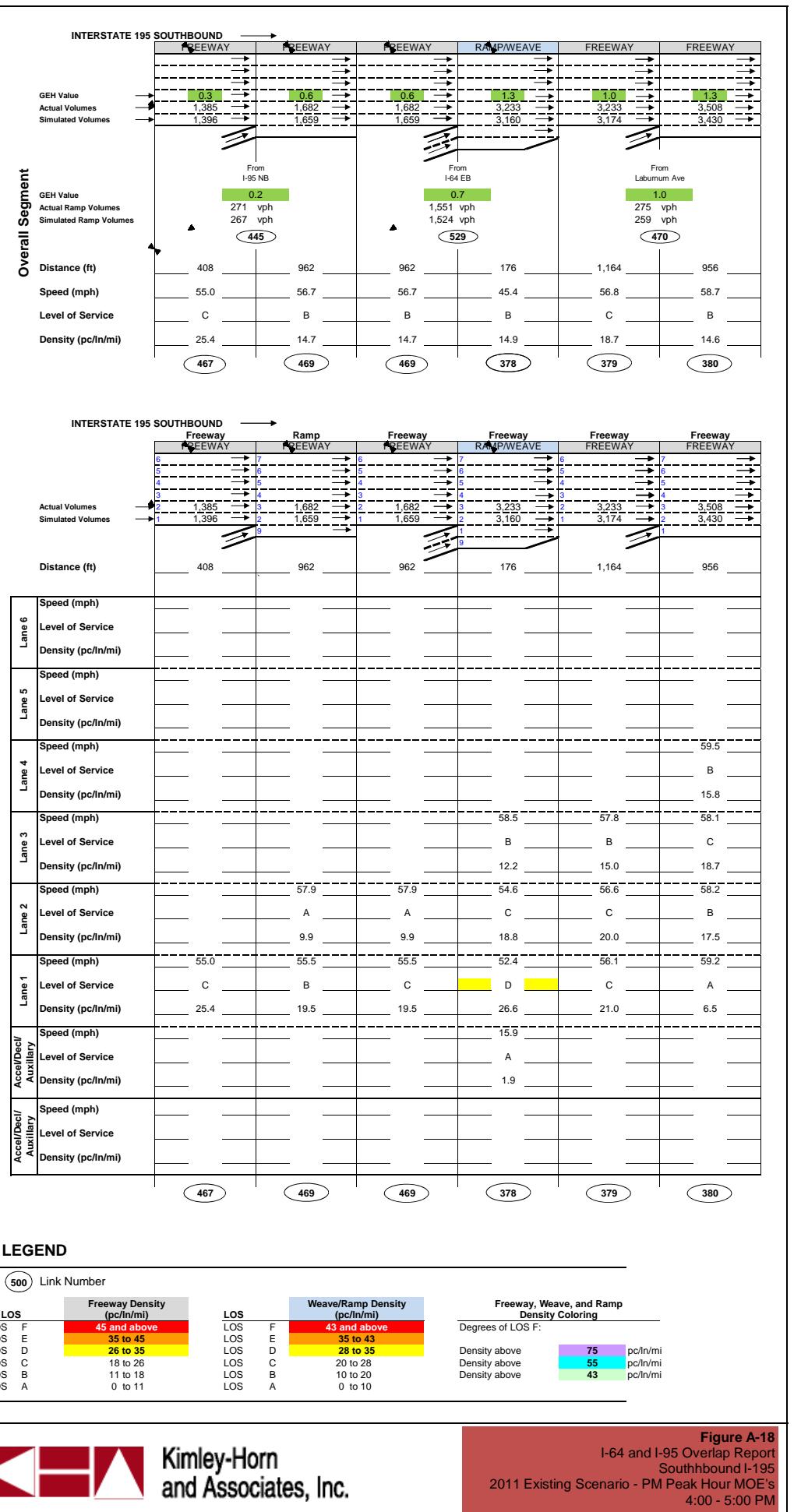
Degrees of LOS F:

F	43 and above
E	35 to 43
D	28 to 35
C	20 to 28
B	10 to 20
A	0 to 10

Density above 75 pc/in/m

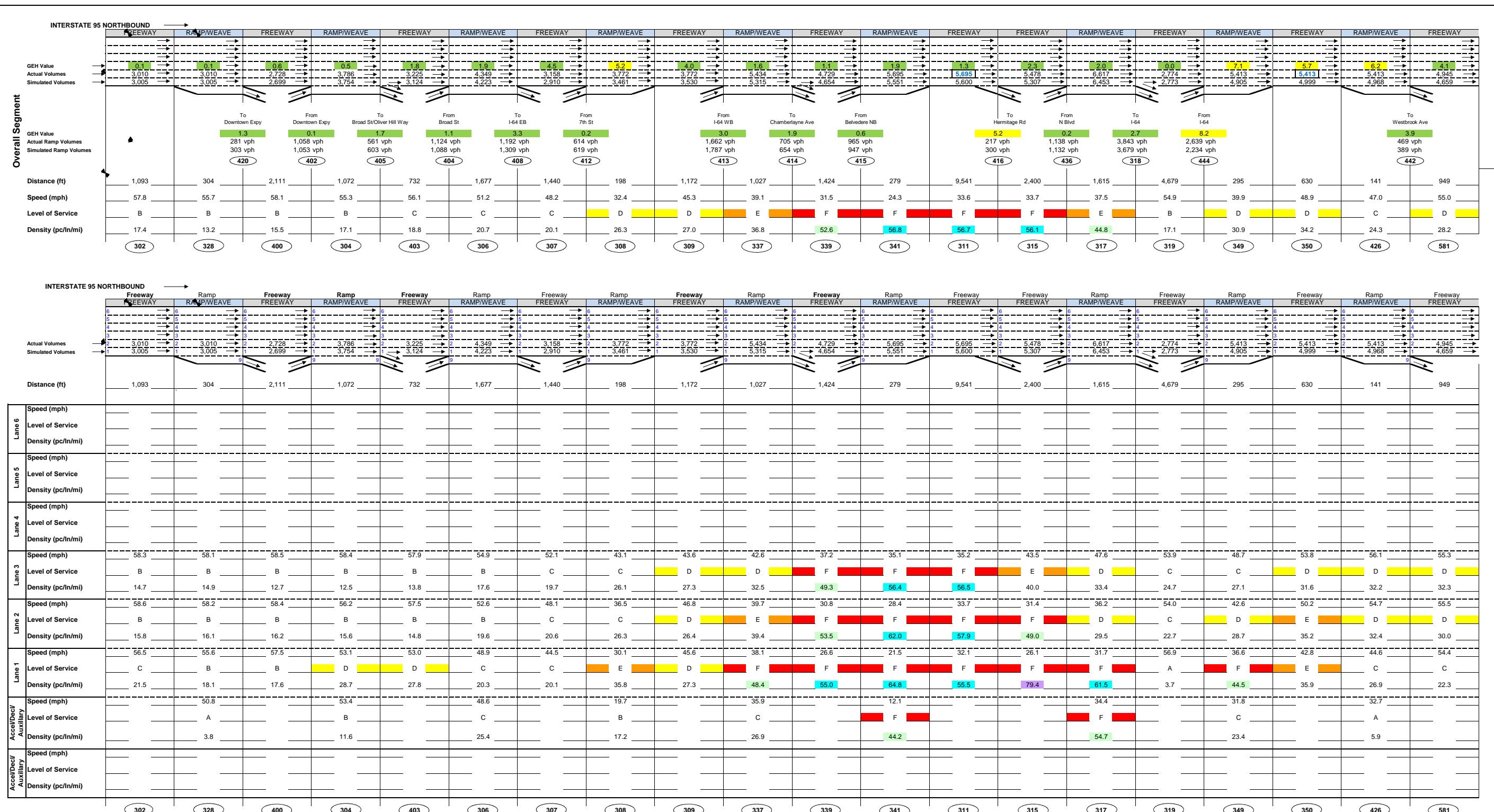
Density above 55 pc/in/m

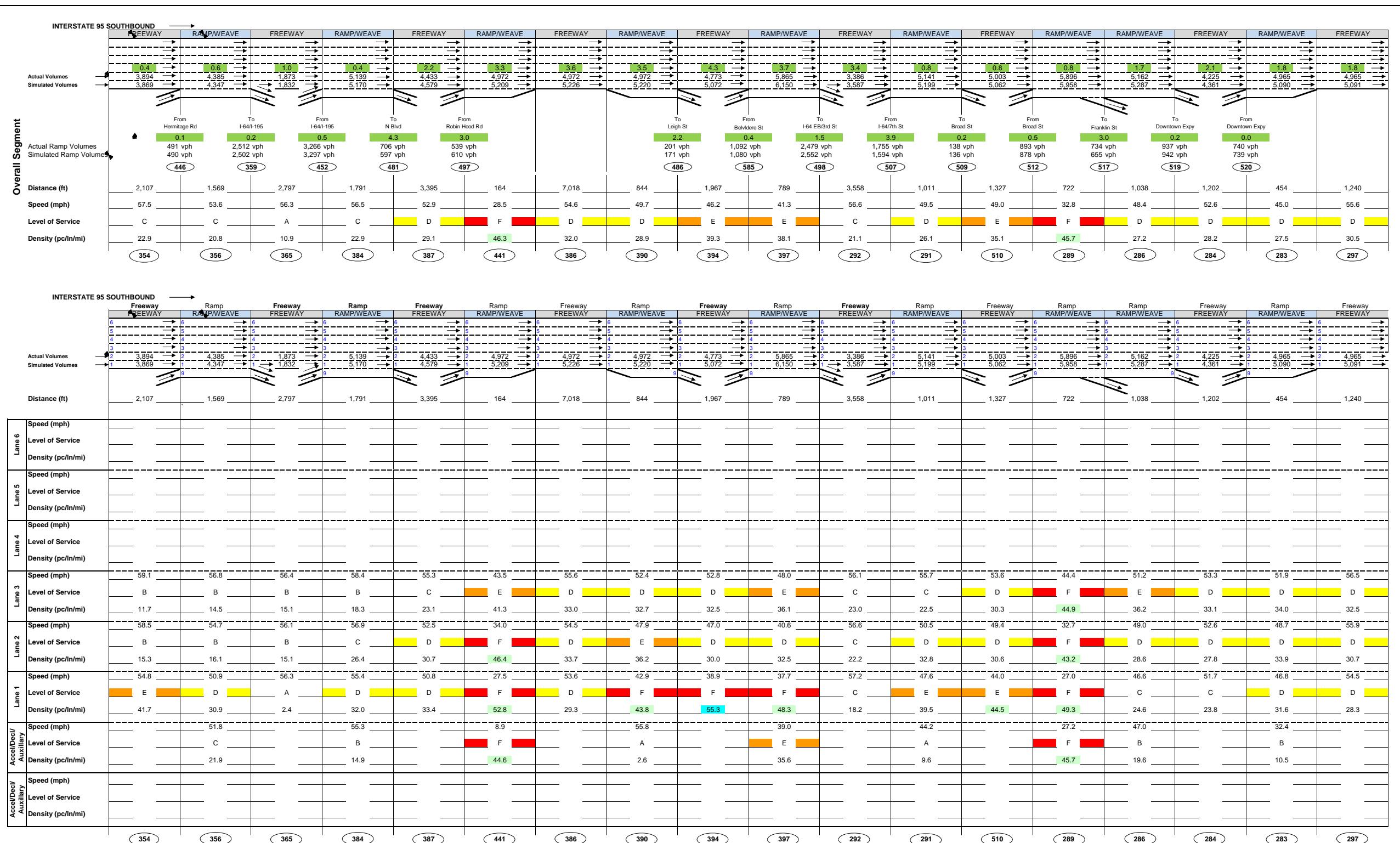
Density above 43 pc/in/m



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**Figure A-18**  
I-64 and I-95 Overlap Report  
Southbound I-195  
2011 Existing Scenario - PM Peak Hour MOE's  
4:00 - 5:00 PM





## LEGEND

500 Link Number		Freeway Density (pc/in/mi)		Weave/Ramp Density (pc/in/mi)		Freeway, Weave, and Ramp Density Coloring	
LOS		LOS		LOS		Degrees of LOS F:	
OS F	45 and above	LOS F	43 and above	LOS E	35 to 43	Density above	75 pc/in/mi
OS E	35 to 45	LOS D	28 to 35	LOS C	20 to 28	Density above	55 pc/in/mi
OS D	26 to 35	LOS B	10 to 20	LOS A	0 to 10	Density above	43 pc/in/mi
OS C	18 to 26						
OS B	11 to 18						
OS A	0 to 11						



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**Figure A-20**  
I-64 and I-95 Overlap Report  
Southbound I-95  
Scenario - PM Peak Hour MOE's  
5:00 - 6:00 PM

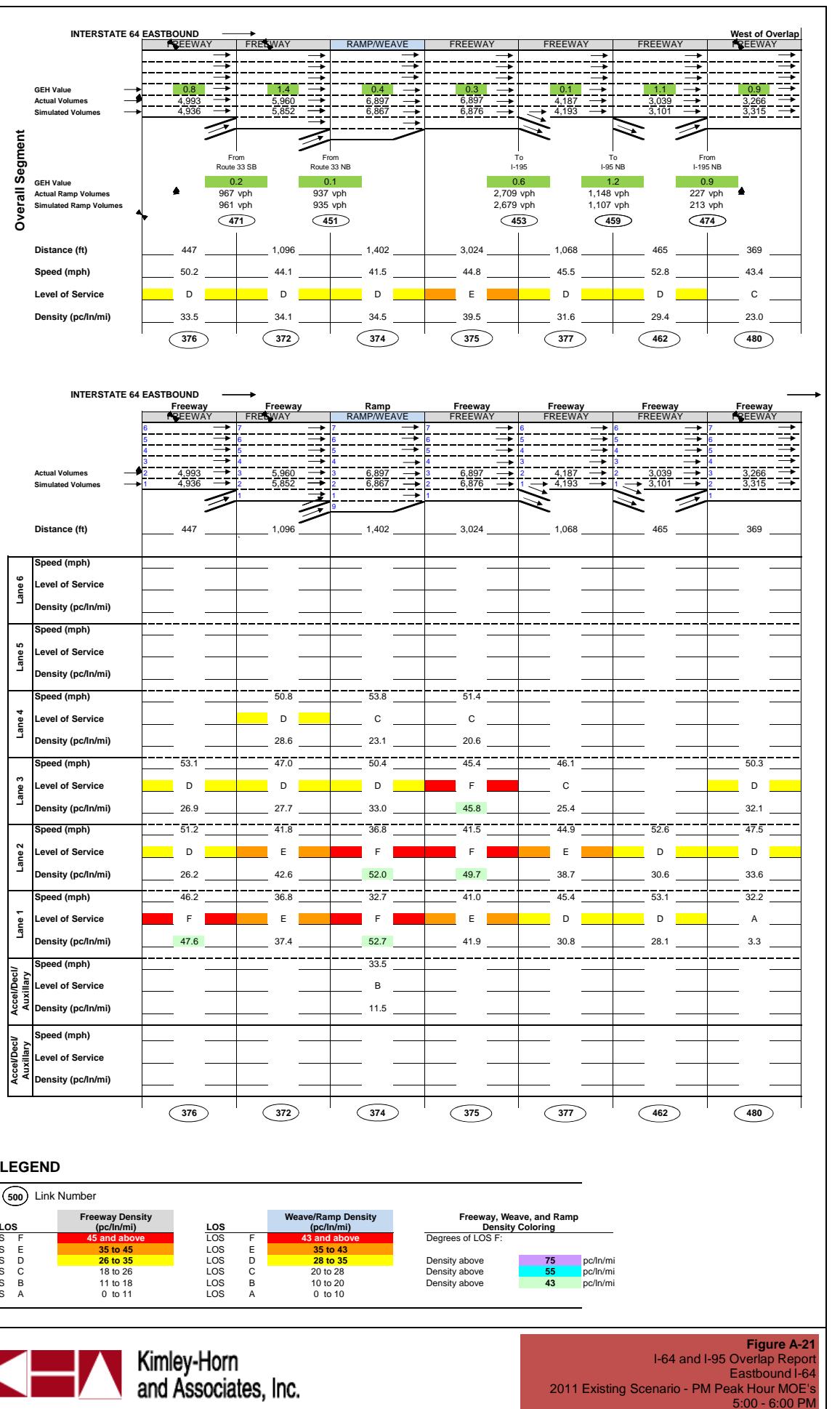
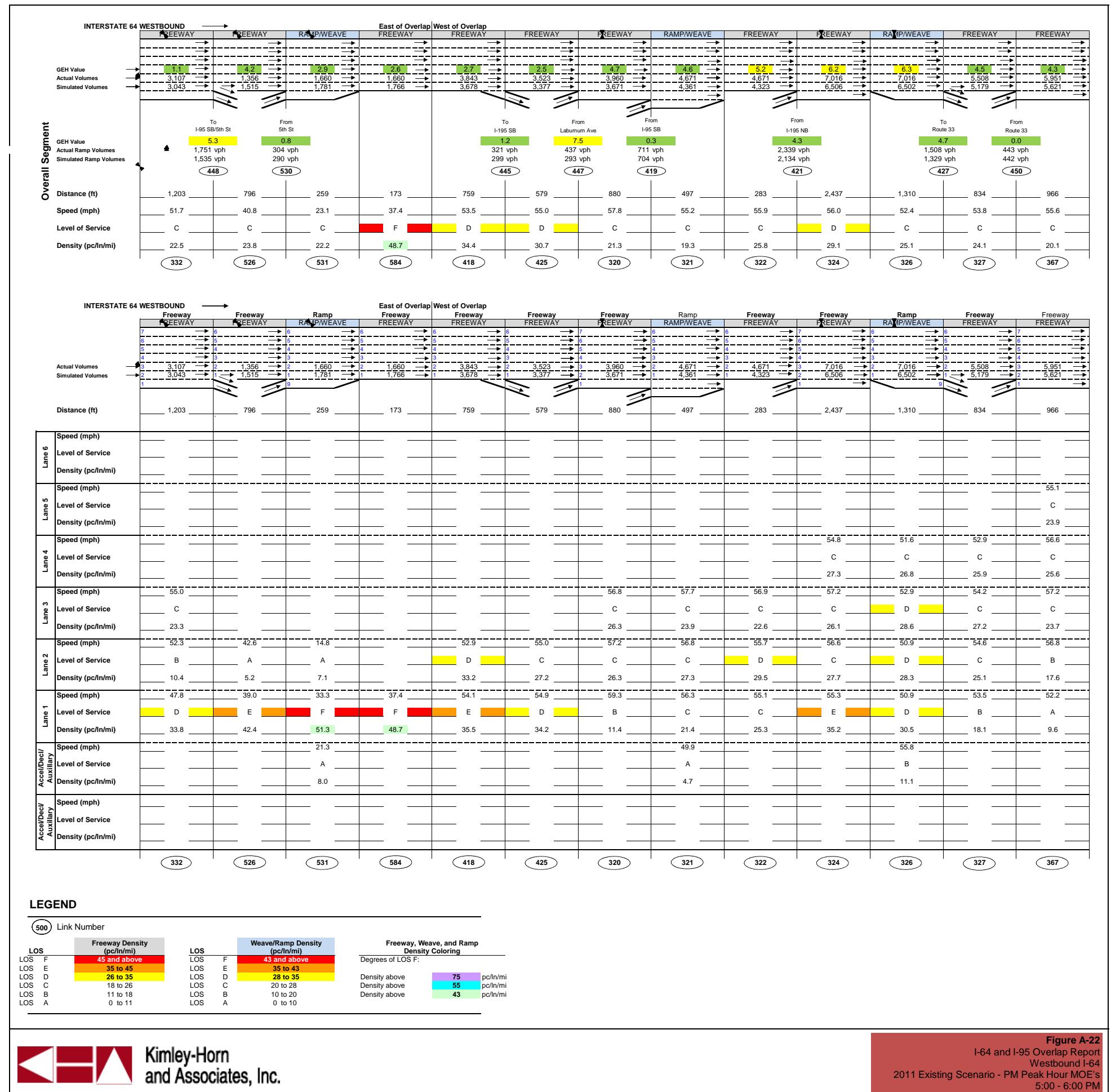
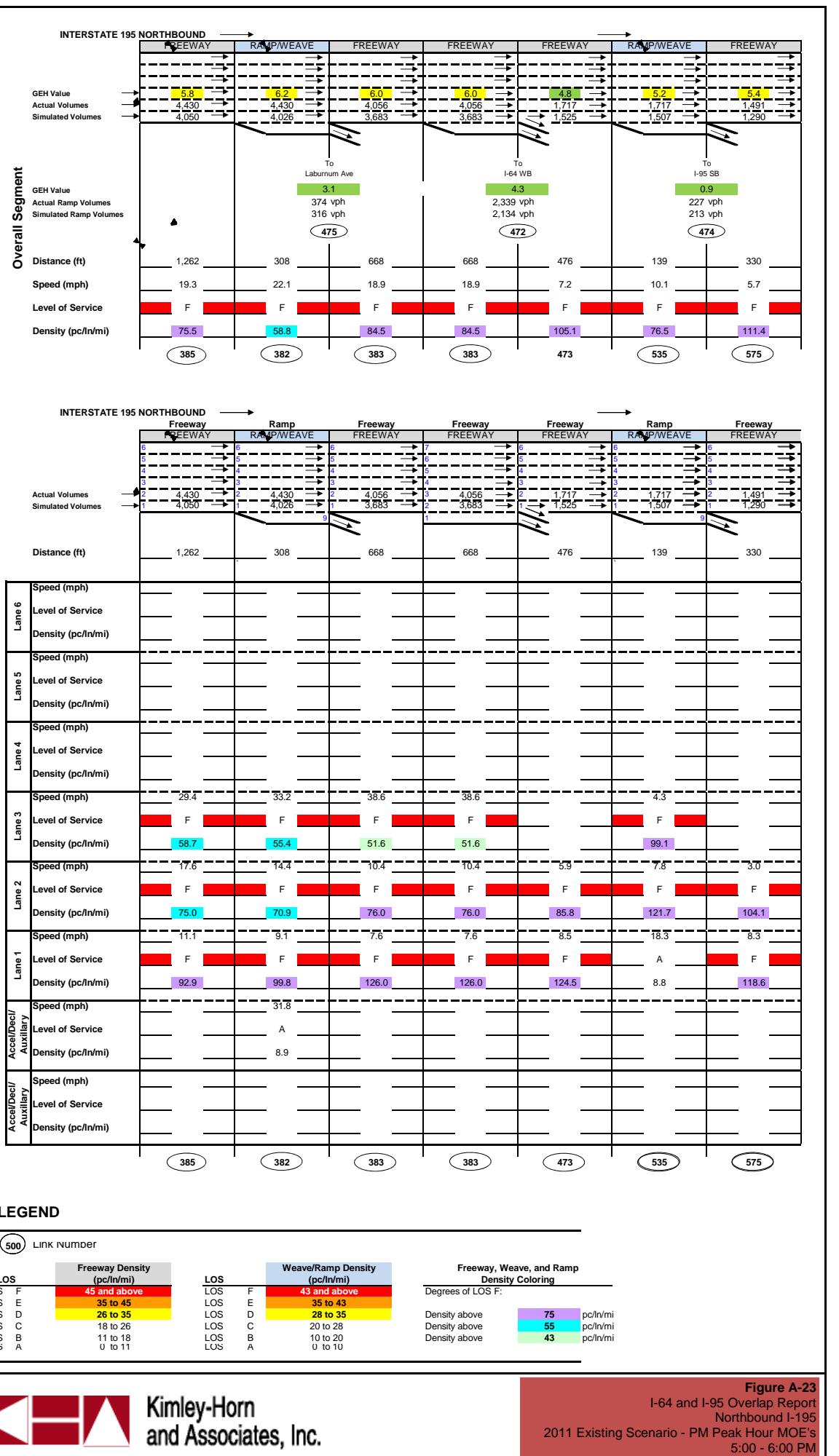


Figure A-21  
I-64 and I-95 Overlap Report  
Eastbound I-64  
2011 Existing Scenario - PM Peak Hour MOE's  
5:00 - 6:00 PM

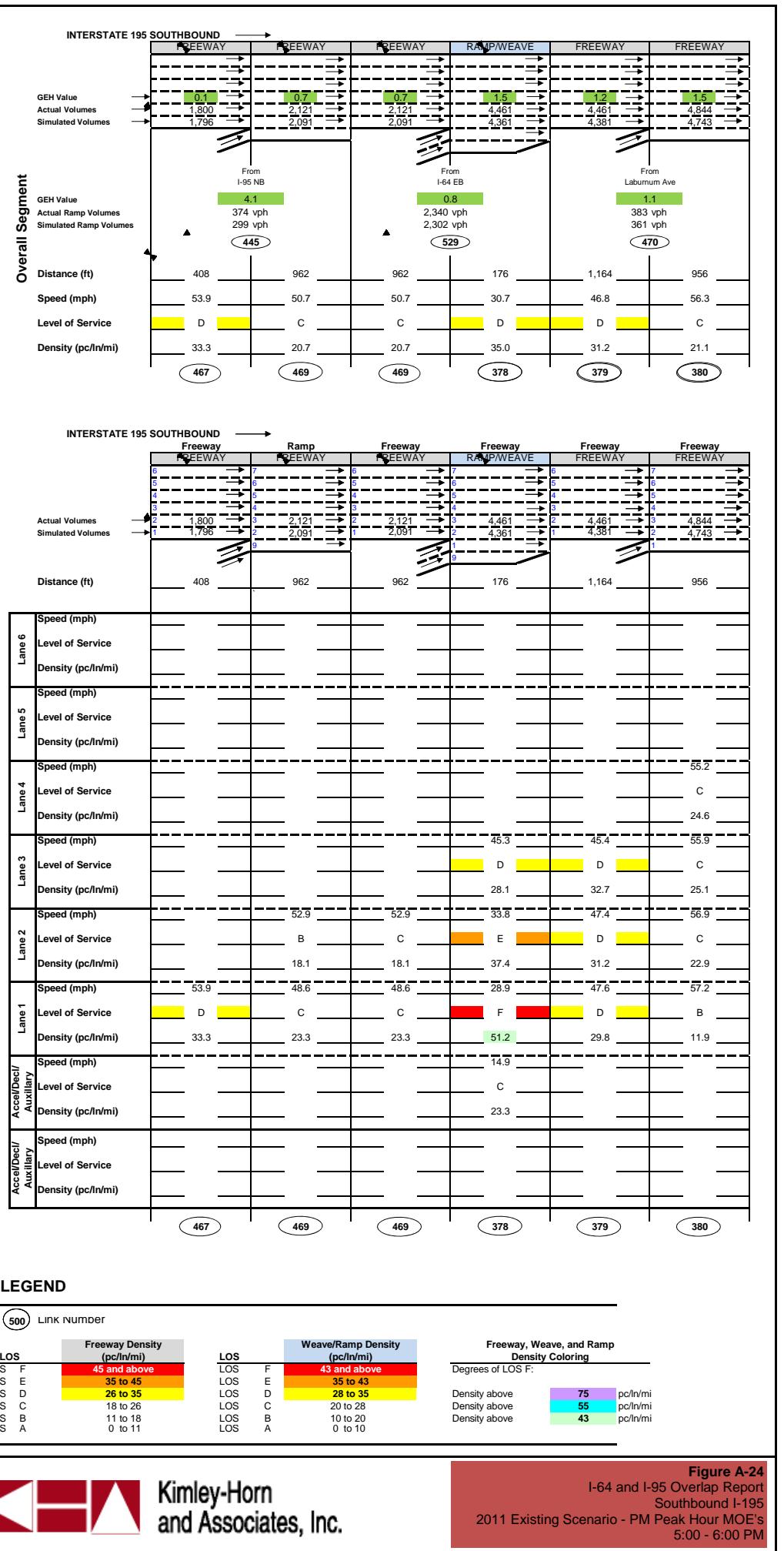


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